#### CITY OF FORT COLLINS TYPE 1 ADMINISTRATIVE HEARING FINDINGS AND DECISION

HEARING DATE:	March 18, 2014
PROJECT NAME:	The Learning Experience at Miramont Office Park Project
CASE NUMBER:	PDP130023
APPLICANT:	Adam Rubenstein Fort Collins Development Co., LLC 5150 East Yale Circle, Suite 400 Denver, CO 80222
OWNER:	Miramont Office Park, LLC 4901 Hogan Drive Fort Collins, CO 80525
HEARING OFFICER:	Kendra L. Carberry

PROJECT DESCRIPTION: This is a request for approval of a Project Development Plan (PDP) for the Learning Experience at Miramont Office Park, with one modification of standard to reduce the size of the outdoor play area. The project is located at 4775 Boardwalk Drive and encompasses two parcels, totaling 70,289 square feet or 1.61 acres. The PDP proposes a 10,000 square feet building and 4,524 square feet of outdoor play area to be used as a Child Care Center.

SUMMARY OF DECISION:	Approved
ZONE DISTRICT:	Harmony Corridor District (H-C)

HEARING: The Hearing Officer opened the hearing at approximately 5:30 p.m. on March 18, 2014, in Conference Room A, 281 North College Avenue, Fort Collins, Colorado.

EVIDENCE: During the hearing, the Hearing Officer accepted the following evidence: (1) Planning Department Staff Report; (2) application, plans, maps and other supporting documents submitted by the applicant (the Land Use Code (the "Code"), the Comprehensive Plan and the formally promulgated polices of the City are all considered part of the record considered by the Hearing Officer).

TESTIMONY: The following persons testified at the hearing:

From the City:	Noah Beals
From the Applicant:	Adam Rubenstein
From the Public:	N/A

#### FINDINGS

1. Evidence presented to the Hearing Officer established the fact that the hearing was properly posted, legal notices mailed and notice published.

2. The PDP complies with the applicable General Development Standards contained in Article 3 of the Code.

a. The PDP complies with Section 3.2.1, Landscaping and Tree Protection, because: the tree mitigation plan was approved by the City Forester; existing street trees will be preserved; and three trees on the property will be relocated.

b. The PDP complies with Section 3.2.2, Access, Circulation and Parking, because: the PDP includes sidewalk connections extending to Boardwalk Drive, to the adjacent office building, and to a pedestrian path connecting with the neighborhood to the west; bicycle parking is provided near the building's main entrance, but separated from the motor vehicle parking; the parking and circulation was already established, with the only improvements being new crosswalk striping in the drive aisle, which will alleviate potential vehicle and pedestrian conflicts; and the requirements for parking spaces and dimensions are satisfied.

c. The PDP complies with Section 3.2.4, Site Lighting, because: the photometric plan complies with the average minimum foot-candles for parking areas and building surrounds; and the one type of light fixture being added to the site includes a light source that is down directional and fully shielded.

d. The PDP complies with Section 3.4.1, Natural Habitats and Features, because the property does not include any natural areas, habitats, or features within, or 500' outside of its boundaries.

e. The PDP complies with Section 3.5.3, Institutional and Commercial Buildings, because: the building provides a play yard, an increase in the number of trees and variety in landscaping, all compensating for the lack of compliance with the 15' of the right-of-way build-to-line requirement; the building elevations provide a recognizable base and top treatment; and the overall design provides variation in massing.

f. The PDP complies with Section 3.6.3, Street Pattern and Connectivity Standards, because the PDP maintains the two vehicle access points to Boardwalk Drive and the drive aisle internal to the site.

g. The PDP complies with Section 3.6.4, Transportation Level of Service Requirements, because the vehicular, pedestrian and bicycle facilities are consistent with the standards contained in Part II of the City's Multimodal Transportation Level of Service Manual.

3. The PDP complies with the applicable standards contained in Article 4 of the Code for the H-C zone district.

a. The PDP complies with Section 4.26(B), Permitted Uses, because a child care center is a permitted use in the H-C zone district.

b. The PDP complies with Section 4.26(D)(3)(a), Land Use Standards, because the proposed building is 1 story in height, below the 6-story maximum.

c. The PDP complies with Section 4.26(E)(1)(b), Development Standards, because the PDP complies with the H-C District Plan and the H-C Design Standards.

4. The Modification of Standard meets the applicable requirements of Section 2.8.2(H) of the Code.

a. The Modification would not be detrimental to the public good.

b. The PDP will promote the general purpose of Section 3.8.4(A) equally well or better than a plan which complies with Section 3.8.4(A), because the proposed outdoor play area provides enough square footage for those children using the area at one time.

c. The standard is intended to provide enough outdoor play area for the children who are using the area at one time. Although the center will have a capacity of 170 children, the applicant has stated that at most only 56 children would be using the outdoor play area at one time, which is equal to the 33% of the total capacity of the center. The applicant's proposal to provide 75 square feet of outdoor play area for 33% of the total child capacity is sufficient outdoor play space tailored to the applicant's specific operations.

#### DECISION

Based on the foregoing findings, the Hearing Officer hereby enters the following rulings:

1. The PDP and Modification of Standard are approved as submitted.

DATED this 26<sup>th</sup> day of March, 2014.

nara barberry

Kendra L. Carberry Hearing Officer



ITEM NO <u>PDP130023</u> MEETING DATE <u>March 18<sup>th</sup>, 2014</u> STAFF <u>Noah Beals</u> ADMINISTRATIVE HEARING OFFICER

### **STAFF REPORT**

- **PROJECT:** The Learning Experience at Miramont Office Park Project Development Plan, PDP130023,
- APPLICANT: Adam Rubenstein, Fort Collins Development CO. LLC 5150 East Yale Circle, Suite 400 Denver, CO 80222
- OWNER: Miramont Office Park LLC 4901 Hogan Drive Fort Collins, CO 80525

#### **PROJECT DESCRIPTION:**

This is a request for approval of a Project Development Plan (PDP) for The Learning Experience at Miramont Office Park. The project is located at 4775 Boardwalk Drive and encompasses two parcels. Together the two parcels are 70,289 square feet or 1.61 acres.

The project proposes to erect a 10,000 square feet building and landscape 4,524 square feet of outdoor play area to be used as a Child Care Center. The property is zoned Harmony Corridor District (H-C). The Child Care Center will be able to accommodate 170 kids and 30-35 employees. A previously installed parking lot contains 73 spaces and the project will provide an additional 4 bike parking spaces. Direct connections to the established sidewalks both in the public right-of-way and on the adjacent lot will be installed. The proposed use is permitted in this zone district. In addition the proposal includes a modification request to allow a reduction of outdoor play area.

**RECOMMENDATION:** Staff recommends approval of The Learning Experience at Miramont Office Park Project Development Plan, PDP130023, and Modification of Standard to Section 3.8.4(A).

#### **EXECUTIVE SUMMARY:**

The approval of The Learning Experience at Miramont Office Park Project Development Plan complies with the applicable requirements of the City of Fort Collins Land Use Code (LUC), more specifically:

- The Learning Experience at Miramont Office Park PDP complies with process located in Division 2.2 – Common Development Review Procedures for Development Applications of Article 2 – Administration.
- The Learning Experience at Miramont Office Park PDP is in conformance with Oak/Cottonwood Amended Overall Development Plan approved by the Planning and Zoning Board in March 1997.
- The Learning Experience at Miramont Office Park PDP complies with relevant standards located in Article 3 General Development Standards, provided that the Modification of Standard to section 3.8.4(A) that is proposed with this project is approved.
- The Learning Experience at Miramont Office Park PDP complies with relevant standards located in Division 4.26, Harmony Corridor District (H-C) of Article 4 – Districts.

#### COMMENTS:

#### 1. Background:

Historically the following approvals have been granted to the property:

- Keenland Annexation, City Council August 1980
- Oak/Cottonwood ODP, Planning and Zoning Board October 1988
- Amendment to Oak/Cottonwood ODP June 1992
- Amendment to Oak/Cottonwood ODP March 1997
- Miramont Office Park, Planning and Zoning Board September 1997
- Miramont Office Park, Final Plan recorded February 1998

Today the property is partially developed. The parking lot has already been installed and public right of way improvements were constructed. The building area is vacant land vegetated with natural grass.

#### Zoning History (most recent to past):

• The property is currently located in the Harmony Corridor District (H-C). The current H-C District was adopted in 1997 at the time the Land Use Code was adopted and the entire City was rezoned. It should be noted that the Harmony Corridor Plan and the Harmony Corridor Design Standards and Guidelines were adopted in March of 1991.

• Prior to the adoption of the Land Use Code and the rezoning to the H-C District, the property was zoned Residential Planned (rp) conditional zoning. The rp zone district was adopted at the time of the Keenland Annexation in 1980. This zone district was in accordance with the adopted Zoning Code at the time.

The current surrounding zoning and land uses are as follows:

Direction	Zone District	Existing Land Use
North	Harmony Corridor (H-C)	Commercial: Business Offices
South	Harmony Corridor (H-C)	Vacant parcel: Part of the Miramont Self Storage PUD approved for offices
East	Harmony Corridor (H-C)	Commercial and Public Right-of-Way: Boardwalk Drive and Sam's Club
West	Harmony Corridor (H-C)	Place of Worship: Front Range Baptist Church and the church's open space

#### 2. <u>Compliance with Article 4 of the Land Use Code – Harmony Corridor (H-C)</u>:

The project complies with all applicable Article 4 standards as follows:

- A. <u>Section 4.26(A) and (B) Permitted Uses</u>
  - The project's proposed Child Care Center use is aligned with the purpose of the Harmony Corridor District to create a complete mixeduse area with a strong employment base. The Child Care Center is a permitted use in Harmony Corridor district.
- B. <u>Section 4.26(D) Land Use Standards</u>
  - Section 4.26(D)(3)(a) establishes a maximum height of six (6) stories the proposed building complies with this standard at height of one (1) story.
- C. <u>Section 4.26(E) Development Standards</u>
  - Section 4.26(E)(1)(b) requires that compliance with the adopted Harmony Corridor District Plan and the Harmony Corridor Design Standards. The proposed development is in compliance with both documents.

 Section 4.26(E)(2)(a) promotes, in cases of multiple parcel ownership, that development plans establish integrated building styles and land uses. The design proposal of the new building is in sync with the other buildings in the office park that are 1 to 2 story, hipped and gable roofs, and consist of stucco with a stone wainscot.

#### 3. <u>Compliance with Article 3 of the Land Use Code – General</u> <u>Development Standards</u>

The project complies with all applicable General Development Standards; with the following relevant comments.

#### A. <u>Division 3.2 – Site Planning and Design Standards</u>

- 1) 3.2.1 Landscaping and Tree Protection:
  - A detailed tree mitigation plan is provided with this PDP. This plan was designed with the coordination and has received approval by the City Forester. In effort to not lose any of the existing tree canopy the existing street trees will be preserved and 3 trees within the site will be relocated.
  - "Full Tree Stocking" is provided along all four sides of the building.
- 2) 3.2.2 Access, Circulation and Parking:

By design the Land Use Code encourages patrons of the site to explore other modes of transportation than the vehicle. This is accomplished by requiring sidewalk connections, bicycle accommodations, and limiting the number of off-street vehicle parking spaces for a non-residential use. The proposed project is in compliance of these standards through the following:

- Sidewalk connections are proposed to extend to Boardwalk Drive, to the adjacent office building, and to a pedestrian path that connects to the neighborhood to the West. As required, the sidewalk system provided contributes to the attractiveness of the development.
- Bicycle parking is provided on site near the building's main entrance and is separated from the vehicle parking. These bike spaces can be accessed through the sidewalk connections or the driveways that lead into the site.

- The parking and circulation was already established. Only improvements proposed are two crosswalk striping's in the drive aisle to help alleviate any impacts from vehicle and pedestrian conflicts. Parking requirements in regards to the maximum numbers of spaces and dimensions of stalls are being met.
- 3) 3.2.4 Site Lighting:
  - A photometric plan was submitted for the project. As proposed, the project complies with the average minimum foot-candles for parking areas and building surrounds.
  - Only one type of Light fixture is being added to the site on the outside of the building. This fixture meets the code standards with a light source that is down directional and fully shielded.
- B. <u>Division 3.4 Environmental, Natural Area, Recreational and Cultural</u> <u>Resource Protection Standards</u>
- 1) 3.4.1 Natural Habitats and Features:
  - The Learning Experience at Miramont site does not include any natural areas, habitats, and features within and 500 feet outside of its boundaries.
- C. <u>Division 3.5 Building Standards</u>
- 1) 3.5.3 Institutional and Commercial Buildings
  - Although the proposed building does not meet the required build-to-line to be within 15 feet of the right-of-way it complies with the exception to the standard allowed by this section. By providing a play-yard, an increase of trees, and variety in landscaping in-between the building and the right-of-way it provides interest and comfort for pedestrians.
  - The proposed building elevations provide a recognizable base and top treatment in accordance with Section 3.5.3(D)(6).
  - The overall design satisfies the institutional building requirements of Section 3.5.3. "Variation in massing". This is satisfied through the use of building projections over the primary entrances and with sloping roofs that provide a variation in height.
- D. <u>Division 3.6 Transportation and Circulation</u>

- 1) 3.6.3 Street Pattern and Connectivity Standards:
  - The project continues to comply with the general framework established with the Overall Development Plan. The project maintains the two vehicle access points to Boardwalk Drive and the drive aisle internal to the site.
- 2) 3.6.4 Transportation Level of Service Requirements:
  - Traffic Operations have reviewed the Transportation Impact Study that was submitted to the City and have determined that the vehicular, pedestrian and bicycle facilities proposed with this PDP are consistent with the standards contained in Part II of the City of Fort Collins Multi-modal Transportation Level of Service Manual.

#### 4. <u>Modification of Standards – Division 2.8</u>

There is one request of modification with this project.

A. <u>Modifcation Request</u>

The applicant requests a modification of standard to Sections 3.8.4(A) to allow a decrease in the amount of square footage required for a Child Care Center. The request is to provide 4,524 square feet. This is 3,226 square feet less than the standard.

- 1) The Standard
  - 3.8.4(A) A minimum of two thousand five hundred (2,500) square feet of outdoor play area shall be provided for fifteen (15) children or fewer, with seventy-five (75) additional square feet being required for each additional child, except that the size of the total play area need only accommodate at least fifty (50) percent of the capacity of the center, and that such outdoor play area shall not be required for drop-in child care centers...
  - The following table illustrates the required square footage for the outdoor play area for The Learning Center at Miramont Office Park.

Calculation of Required Square Footage Per Land Use Code				
The minimum square footage for outdoor Play Area for 15 children or fewer.	2,500 Sq. Ft.			
The Learning Center at Miramont Office Park Capacity	170 Children			
50% of the Capacity of Center	170/2=85 Children			
An additional 75 sq. ft. For every additional child above the initial 15 of half the Capacity	85-15=70 Children 70x75=5,250 Sq. ft.			
Total square footage required	2,500+5,250=7,750 Sq. Ft.			

#### 2) Section 2.8 Code Criteria

The request of approval for this modification is based on the Review Criteria for Modification of Standards found in Section 2.8.2(H) and 2.8.2(H)(1) as follows:

- The granting of the modification would not be detrimental to the public good.
- The plan as submitted will promote the general purpose of the standard for which the modification is requested equally well or better than a plan which complies with the standard for which a modification is requested.

#### 3) Applicant's Justification

The applicant has provided the following justification for the modification request:

- The reduced playground size, 4,524 sf complies with both Federal and Colorado State licensing codes.
  - According to Colorado State regulations, 12 CCR 2509-8 Program Area 7-Child Care Center/less than 24 Hour Care, section 7.702-7.702.104 goes over space requirements. Specifically, for outdoor play area requirements, the State requires us to have a minimum of 75 square feet of space per child for a group of children using the total play area at any one time. The total square footage must accommodate

at least 33% of the licensed capacity of the center or a minimum of 1,500 sf, whichever is greater.

- We are anticipating a capacity of 170 children. Based on the above requirements, we will have a maximum of 56 children outside at one time. Based on the State requirements, we will need to have 4,200 sf of space for our children. As we want to build as much outdoor space as possible, we are building an additional 324 sf of playground space; thus a 4,524 sf playground.
- Further, the way The Learning Experience operates their outside areas (and per Colorado State Code and Regulations) is play time is determined by age groups. We do not have 1-2 year old children outside with 4-5 year old children. The reduced playground size helps our staff and educators supervise the children outside to ensure everyone's safety. We do not feel a 7,750 sf playground is necessary with the limited amount of children that will be outside at any given time as we limit the play time per the age groups. With a larger playground and less children, there is more room for errors and mistakes to be made as there are not enough sets of eyes supervising the children; a larger space increases the likely hood of injury and/or mischief.
- Another factor in asking for a modification request is the land limitations. We are taking the fifth and last parcel of a planned development. Per the site plan, we only have so much land we can build on without effecting the entire development. To the north and west of the site, parking fields have already been created. We cannot take away parking spaces for the four other buildings, and our use, and still meet code.
  - To the south and east of the site, there is a detention pond. We are able to encroach to the east of the detention pond, by drudging out the pond to the south to make up for lost capacity. However, we still need enough capacity for not only our building and the four other buildings in the office park, but also the church that is to the northeast of our property. We physically cannot get a 7,750 sf playground on this location; however, based on a 4,524 sf playground, we still meet Colorado State regulations for playground size without being detrimental to our neighboring buildings.

4) Staff Finding for the Modification

Staff finds that the request for Modification of Standard to Section 3.8.4(A) is justified by the applicable standards in 2.8.2(H). The granting of the Modifications would not be detrimental to the public good and:

- The request satisfies Criteria 2.8.2(H)(1) because the plan as submitted will promote the general purpose of the standard equally well or better than a plan which complies with the standard based on the justification statement provided by the applicant and in addition:
  - The standard is intended to provide enough outdoor play area for the children who are using this area at one time. Although the center has a capacity of 170 children, the applicant has stated that at most only 56 children would be using the outdoor play area at one time, which is equal to the 33% of the total capacity of the center. The applicant's proposal to provide 75 square feet of outdoor play area for 33% of the total child capacity is sufficient outdoor play space tailored to their specific operations.

#### 5. <u>Findings of Fact/Conclusion</u>

In evaluating the request for The Learning Experience at Miramont Office Park Project Development Plan, Staff makes the following findings of fact:

- A. The Modification of Standard to Section 3.8.4(A) regarding the decrease in required outdoor play area that is proposed with this PDP would not be detrimental to the public good and the modification meets the applicable requirements of Section 2.8.2(H)(1). The proposed plan will promote the general purpose of the standard equally well or better than a plan which complies with the standard due to the proposed outdoor play area provides enough square footage for those children using the area at one time.
- B. The Learning Experience at Miramont Office Park PDP complies with process located in Division 2.2 Common Development Review Procedures for Development Applications of Article 2 Administration.
- C. The Learning Experience at Miramont Office Park PDP is in conformance with Oak/Cottonwood Amended Overall Development Plan approved by the Planning and Zoning Board in March 1997.

- D. The Learning Experience at Miramont Office Park PDP complies with relevant standards located in Article 3 General Development Standards, provided that the Modification of Standard to Section 3.5. is approved.
- E. The Learning Experience at Miramont Office Park PDP complies with relevant standards located in Division 4.28, Harmony Corridor District (H-C) of Article 4.

#### **RECOMMENDATION:**

Staff recommends approval of The Learning Experience at Miramont Office Park Project Development Plan, PDP1300023.

#### ATTACHMENTS:

- 1. Statement of Planning Objectives
- 2. Oak/Cottonwood Farm Amended Overall Development Plan
- 3. Site Plan
- 4. Landscape Plans
- 5. Building Elevations
- 6. Lighting Plan
- 7. Traffic Impact Statement

Dear City of Fort Collins,

**Rubenstein Real Estate, Co., LC,** on behalf of **Fort Collins Development Co., LLC**, has contracted to buy **0.237 acres** of land at **The Miramont Office Park**, Lot 3, Pad E; Larimer County; to build a **10,000 SF** "Child Development Facility" with a contiguous **5,000 SF** playground for The Learning Experience "TLE".

**TLE** is a privately held company headquartered in **Boca Raton**, **Florida**. TLE focuses on early childhood development for children aging from **6 weeks to 6 six years old**, and after school care for children up to 9 years old.

TLE currently has **120** operating facilities and **60** facilities under development in **22** states. This facility will be the **fifth** in Colorado.

Please visit TLE at their website: www.thelearningexperience.com

The development of Pad E in Lot 3 includes constructing a 10,000 SF building, a contiguous 5,000 SF playground, and fully landscaping the pad.

TLE will be an outstanding addition to the Miramont Office Park, and the surrounding neighborhoods. The facility will allow enrollment up to **170 children** and will have a staff of approximately 30. TLE provides the tools and environment that gives children every opportunity to develop to their greatest potential. TLE has a cutting edge proprietary curriculum; over **88% of TLE's 4-year old children** are reading. TLE builds a strong foundation for each child to grow emotionally, socially, and cognitively at his/her own pace.

Adam Rubenstein represents the applicant and is the "Single Point of Contact" for the application; his information is below:

Adam Rubenstein	Office: 720-529-2881
5150 East Yale Circle, Suite 400	Cell: 303-257-1481
Denver, CO 80222	arubenstein@legendretailgroup.com

#### Statement of Planning Objectives (D)

- (i) This project is an infill commercial development, which the City of Fort Collins supports.
- (ii) The Learning Experience will be taking the last pad in the already existing Miramont Office Park. We will have a 10,000 sf building with a contiguous 5,000 sf playground. The playground will be encompassed by an opaque fence that has landscaping surrounding the exterior. There is an onsite detention pond.

- (iii) The Miramont Office Park consists of five pads and five buildings. Each building has a separate owner. The owners are all a part of the Declaration of Covenants, Conditions, and Restrictions for the Miramont Office Park dated October 11, 2000 reception # 2000070367 to govern and maintain the office park and common areas. Pad E will be owned by the Fort Collins Development Co, LLC which is controlled and Managed by John and Adam Rubenstein. Pad E will be a part of the CCR's mentioned above.
- (iv) The Learning Experience will be licensed for roughly 170 children. The Learning Experience will employ an estimated 30-35 employees.
- (v) The Learning Experience chose the Miramont Office Park for its Ft. Collins home because of its location off of Harmony Rd and Boardwalk. It is next to several businesses and located in an established, yet growing community. The Learning Experience building has been designed to match the rest of the office park. We will have a 5,000 sf playground, with an opaque fence. We are using an opaque fence for security reasons. We will have landscaping surrounding the fence to help camouflage the fence.
- (vi) Child Care is a permitted use under the current zoning restrictions.
- (vii) We are the fifth and last piece to an already existing office park. The City supports infill projects.
- (viii) No neighborhood meeting has taken place.
- (ix) The Learning Experience at Miramont Office Park
- (E) Attached hereto
- (F) No new street names are being proposed
- (G) Attached hereto
- (H) Gene Vaughan 970.226.3990 work 970.227.5968 mobile genev@remax.net
  - Gary Nordic 970.217.1742 mobile gary@garynordic.com
  - Ron Young 970.481.0808 mobile ronyoung@frii.com

#### (I) John Rubenstein

6310 Lamar, #220 Overland Park, KS 66202 913.362.1999

#### Adam Rubenstein

5150 East Yale Circle, suite 400 Denver, CO 80222 303.257.1481

#### (J) Construction schedule

Break ground January/February 2014 Site/utility work 30-45 days 4 months to build the building and furnish the interior Anticipate an August 2014 opening to the public.

#### 13-16 Not applicable for this project.



ENGINEERING DEPT. NOTE: THIS REPRESENTS THE BEST QUALITY IMAGE POSSIBLE TAKEN FROM VERY POOR QUALITY

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# PROJECT DEVELOPMENT PLAN FOR THE LEARNING EXPERIENCE AT MIRAMONT OFFICE PARK

### **PROJECT TEAM**:

DEVELOPER **RUBENSTEIN REAL ESTATE** C\O LEGEND RETAIL GROUP 5150 E. YALE CIRCLE, SUITE 400 DENVER, COLORADO 80222 720.529.2881 CONTACT: ADAM RUBENSTEIN

### **CIVIL ENGINEER** REDLAND 8000 LINCOLN STREET, SUITE 206

LITTLETON, COLORADO 80122 720.283.6783 CONTACT: MARK CEVAAL, P.E. EMAIL: MCEVAAL@REDLAND.COM

#### LANDSCAPE ARCHITECT BRITINA DESIGN GROUP 7600 GRANDVIEW AVE., SUITE 210 ARVADA, COLORADO 80002 303.556.2887 CONTACT: CHRIS COOPER

ARCHITECT

LIMA ARCHITECTS 4855 TECHNOLOGY WAY, SUITE 200 BOCA RATON, FLORIDA 33431 561.886.6400 CONTACT: OCTAVIO LIMA

### LEGAL DESCRIPTION

LOT 3 OF THE MIRAMONT OFFICE PARK LOCATED IN THE NORTHEAST QUARTER OF SECTION 1, TOWNSHIP 6 NORTH, RANGE 69 WEST OF THE SIXTH PRINCIPAL MERIDIAN, CITY OF FORT COLLINS, COUNTY OF LARIMER, STATE OF COLORADO

LOT 3 WITHIN THE MIRAMONT OFFICE PARK PUD A PLANNED UNIT DEVELOPMENT IN THE CITY OF FORT COLLINS COUNTY OF LARIMER, STATE OF COLORADO OCTOBER 2013



VICINITY MAP 1"=2000'

### BASIS OF BEARINGS

THE NORTH LINE OF THE NORTHEAST QUARTER OF SECTION 1 TOWNSHIP 6 NORTH, RANGE 69 WEST OF THE SIXTH PRINCIPAL MERIDIAN IS CONSIDERED TO BEAR NORTH 89°58'54" EAST.

### B**ENC**H**MA**RK

NGS CONTROL POINT Y402 BEING A STAINLESS STEEL ROD IN A RANGE BOX ON EAST SIDE OF HIGHWAY 287, 900± SOUTH OR HARMONY ROAD. PUBLISHED ELEVATION 5018.96 (NAVD88)

UNADJUSTED NGVD 1929 ELEVATION 5015.76 PER CITY OF FT. COLLINS PLANS ARE BASED ON NAVD88 ELEVATION

INDEX OF DRAWINGS			
SHEET NO.	DESCRIPTION		
C0.0	COVER SHEET		
C1.0	SITE PLAN		
C2.0	GRADING PLAN		

THIS IS TO CERTIFY THAT, AS OF THE \_\_\_\_\_ DAY OF \_, 2013 THE UNDERSIGNED PARTIES ARE SOLE OWNERS OF RECORD OF THE REAL PROPERTY DESCRIBED IN THE ABOVE SITE PLAN AS CONSTRUED IN CRS. 31-23-11AND CONSENT TO THE DEVELOPMENT OF SAID PROPERTY AS SHOWN ACCORDING TO SAID PLAN

#### OWNER

MIRAMONT OFFICE PARK OWNERS ASSN. 4703A BOARDWALK DR. FORT COLLINS, CO 80525

STATE OF COLORADO ) )SS

COUNTY OF LARIMER )

THE FORGOING INSTRUMENT WAS ACKNOWLEDGED BEFORE ME THIS \_\_\_\_\_ DAY OF \_\_\_\_\_, 2013 BY \_\_\_\_\_ \_\_AS

MY COMMISSION EXPIRES WITNESS MY HAND AND OFFICIAL SEAL.

#### NOTARY PUBLIC

APPROVED BY THE PLANNING DIRECTOR OF THE CITY OF FORT COLLINS COLORADO ON THIS \_\_\_\_\_ DAY OF \_\_\_\_A.D. 2013

PLANNING DIRECTOR

	References and the second seco							
	DRAWN ARD	CHECKED MDC	APPROVED MDC	PROJECT NO. 12026			VERT. SCALE N/A	
DATE NO. NOTES	07.12.2013 1 1ST SUBMITTAL	10.04.2013 2 2ND SUBMITTAL	01.31.2014 3 3RD SUBMITTAL					
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LAND USE TABLE					
EX. ZONING HC-HARMONY CORRIDOR					
LOT AREA	1.614 AC; 70,289 SF				
COVEF	RAGE				
BUILDING	10,000 SF; 14%				
PARKING AND DRIVES	29,422 SF; 42%				
LANDSCAPING	23,181 SF; 33%				
HARDSCAPE	7,686 SF; 11%				
PARKING S	UMMARY				
REGULAR SPACES	71				
ACCESSIBLE SPACES	2				
TOTAL PARKING PROVIDED	73				
MAXIMUM BUILDING HEIGHT 24'					
USE: CHILD CA	RE FACILITY				

				Where Great Places Beain	8000 South Lincoln Street #306 Littleton CO 80133		
DRAWN AKD	CHECKED MDC	APPROVED MDC	PROJECT NO. 12026	HORZ. SCALE 1" = 20'	VERT. SCALE n/a		
DATE NO. NOTES 07.12.2013 1 1ST.SUBMITTAI	10.04.2013 2 2ND SUBMITTAL	01.31.2014 3 3RD SUBMITTAL					
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EXISTING TREE TO REMAIN

EXISTING TREE TO BE RELOCATED

SHRUBS & PERENNIALS

EXISTING TURF TO REMAIN

IRRIGATED NATIVE GRASS

STEEL EDGING

## PLANT LEGEND

· <b>·</b> ·					
Sym.	Botanic Name/Common Name	Size	Spacing	Quantity	Hydrozone
Decio	luous Trees				
AA	Amelanchier alnifolia Saskatoon Serviceberry	6' ht.	As Shown	2	Low
AT	Acer tataricum 'Hot Wings' Hot Wings Maple	1.5" cal.	As Shown	3	Moderate
GD	Gymnocladus dioicus 'Espresso' Seedless Kentucky Coffeetree	2" cal.	As Shown	3	Low
GT	Gleditisia triacanthos inermis Imperial Honeylocust	2" cal.	As Shown	1	Moderate
PC	Pyrus calleryana 'Chanticleer' Chanticleer Pear	2" cal.	As Shown	2	Moderate
Decic	luous / Evergreen Shrubs				
CI	Caryopteris x incana 'Dark Knight' Dark Knight Spirea	5 gal.	3' o.c.	28	Low
CS	Cornus sericea Arctic Fire Arctic Fire Dogwood	5 gal.	4' o.c.	30	Moderate
PA	Perovskia atriplicifolia Russian Sage	5 gal.	3' o.c.	27	Low
PB	Prunus besseyi Western Sandcherry	5 gal.	4' o.c.	13	Low
RA	Ribes aureum Yellow Flowering Current	5 gal.	4' o.c.	8	Low
VD	Viburnum dentatum 'Blue Muffin' Blue Muffin Arrowwood Viburnum	5 gal.	4' o.c.	3	Moderate
Orna	mental Grasses				
CAB	Calamagrostis brachytricha Korean Feather Reed	1 gal.	3' o.c.	85	Low
HES	Helictotrichon sempervirens Blue Oats Grass	1 gal.	2' o.c.	63	Low

Scale: 1" = 20'-0"

Native Grass Mix

New Seeding	Dryland: 20-25 lbs/acre
	Irrigated: 40 lbs/acre
Overseeding	Dryland: 10-15 lbs/acre
	Irrigated: 20 lbs/acre

Mix: 30% Ephraim Crested Wheatgrass

25% Sheep Fescue

20% Perennial Rye 15% Chewings Fescue

10% Canada Bluegrass





### LANDSCAPE NOTES

- 1. Landscape materials shall be in accordance with AAN specifications for Number One Grade.
- Turf areas shall be irrigated with an automatic pop-up irrigation system. Shrub beds and 2. trees to be irrigated with an automatic drip (trickle) irrigation system. The irrigation plan shall be reviewed and approved by the City of Ft. Collins prior to the issuance of a building permit.
- 3. Trees to be balled and burlapped, unless otherwise noted.
- Shrub areas shall be mulched with 3"-5" river rock cobble over weed barrier fabric, typ.
- 5. Landscaping must be installed or secured with an irrevocable letter of credit. Performance Bond, or escrow account for 125% of the valuation of the materials and labor prior to issuance of a Certificate of Occupancy.
- 6. Landscaping shall be installed and maintained by the property owner, including trees and groundcover within the R.OW.
- The following separations shall be provided between trees/shrubs and utilities: - 40 feet between canopy trees and street lights
- 15 feet between ornamental trees and streetlights
- 10 feet between trees and public water and sanitary and storm sewer lines - 4 feet between shrubs and public water and sanitary and storm water line
- 4 feet between trees and gas lines
- Field locate utilities prior to planting.
- 9. To the maximum extent feasible, topsoil that is removed during construction activity shall be conserved for later use on areas requiring revegetation and landscaping.
- 10. Perennial and ornamental grass beds to be mulched with 4" depth shredded wood mulch. No weed barrier shall be placed in these beds.
- 11. The top 6 inches of topsoil will be stripped and stored on site. The soil will then be placed in the planting areas.
- 12. The soil in all landscape areas, including parkways and medians, shall be thoroughly loosened to a depth of not less than eight (8) inches and soil amendment shall be thoroughly incorporated into the soil of all landscape areas to a depth of at least six (6) inches by tilling, discing or other suitable method, at a rate of at least three (3) cubic yards of soil amendment per one thousand (1,000) square feet of landscape area.
- 13. A free permit must be obtained from the City Forester before trees or shrubs as noted on this plan are planted, pruned or removed on the public right-of-way. This includes zones between the sidewalk and curb, median and other city property. This permit shall approve the location and species to be planted. Failure to obtain this permit may result in replacing or relocating trees and a hold on certificate of occupancy.
- 14. The developer shall contact the City forester to inspect all tree plantings at the completion of each phase of the development. All trees need to have been installed as shown on the landscape plan. Approval of street tree planting is required before final approval of each phase. Failure to obtain approval by the City Forester for the street trees in a phase shall result in a hold on certificate of occupancy for future phases of the development.

## TREE PROTECTION NOTES

- Within the drip line of any protected existing tree, there shall be no cut or fill over a four (4) inches depth unless a qualified arborist or forester has evaluated and approved the disturbance.
- 2. All protected existing trees shall be pruned to the City of Fort Collins Forestry standards
- Prior to and during construction, barriers shall be erected around all protected 3 existing trees with such barriers to be of orange fencing a minimum of four (4) feet in height, secured with metal T-post, no closer than six (6) feet from the trunk or one-half  $(\frac{1}{2})$  of the drip line, whichever is greater. Thereshall be no storage or movement of equipment, materials, debris or fill within the fenced tree protection zone.
- During the construction stage of development, the applicant shall prevent the cleaning of equipment or material or the storage and disposal of waste material such as paints, oils, solvents, asphalt, concrete, motor oil or any other material harmful to the life of a tree within the drip line of a protected tree or group of trees.
- No damaging attachment, wires, signs or permits may be fastened to any protected tree.
- Large property areas containing protected trees and separated from construction 6 or land clearing areas, road rights-of-way and utility easements may be "ribboned off", rather than erecting protective fencing around each tree as required as required by subsection (G)(3) above. This may be accomplished by placing metal t-post stakes a maximum of fifty (50) feet and tying ribbon or rope from stake-to-stake along the outside perimeters of such areas being cleared.
- The installation duties, irrigation lines or any underground fixture requiring excavation deeper than six (6) inches shall be accomplished by boring under the root system of protected existing trees at a minim depth of tewnty-four (24) inches. The auger distance is established from the face of the tree (outer bark) and is scaled from the tree diameter at breast height as described in the chart below.

Tree Dia. at Breast Height (inch.)	Auger Dist. From Face of Tree (ft.)
0-2	1
3-4	2
5-9	5
10-14	10
15-19	12
Over 19	15

## TREE RELOCATION NOTES

construction. Tree Spade Size

- 44 inches
- 66 inches
- 92 inches 100 inches

120 inches

## large diameter.

- necessary.
- transport the tree.
- operations.
- later.
- with a solid ball of earth.

- suffocate.
- winds and sun exposure.
- and turf.

The following is a general guide for tree sizes and spade sizes depending on the type and condition of the trees to be moved. Contractor to evaluate trees which are slated to be moved and determine which size spade will best accommodate the tree. Contractor to put in writing how they will proceed in transplanting trees including spade size, time, staking, ect. and submit the report to the project manager for approval prior to

Deciduous Tree-Trunk Diameter	Evergreen Tree-Height
2 to 3 inches	5 to 7 feet
3 to 5 inches	7 to 10 feet
6 to 8 inches	12 to 15 feet
8 to 10 inches	17 to 20 feet
10 to 12 inches	20 to 24 feet

Note: Trunk diameters are measured using a caliper, six inches above the ground for tree four inches in diameter or smaller and 12 inches above the ground for trees with a

A. Certain species such as firs, maples, and spruce are best transplanted during the cooler months of spring and early fall. Plants moved in the summer and fall should be dug with an oversized root ball with special attention paid to the species of the tree, its condition and how it is transported. Reduce water loss as much as possible during transportation. Do not move trees on hot, windy days. Spray with an anti-transpirant prior to transplanting to reduce water loss.

B. The soil type of the new location should match the soil type, drainage and pH of the original site as closely as possible. Duplicate the original soil conditions as closely as possible by taking soil analysis at existing tree location and at new location. Add amendments to the new location as recommended by the report, if

**C.** Use the same tree spade to excavate the planting hole as was used to extract and

D. Using a tree spade may also cause glazing, the compaction of the soil in the newly-dug hole, which will affect the lateral movement of soil moisture and root penetration. Contractor to roughing up the sides of the new hole and the plug using a shovel, rake or fork, cut exposed roots cleanly during transplanting

E. Water the tree thoroughly 1 to 2 days prior to digging to keep the ball intact and reduce as much soil loss as possible during transport. When positioning the tree in the new hole, it should be placed 2-3" higher than the original grade to allow for settling and orient in the same direction as its original location when possible. If a tree is planted too shallow, the roots may be damaged by temperature fluctuations and lack of soil moisture. Planting a tree too deeply will result in girdling problems

F. When extracting the tree, center the trunk within the tree spade and move tree

G. Contractor to stake tree securely after transplanted taking into consideration the wind direction. Contractor to submit in writing the type and size of stakes, the strapping, wiring, turnbuckle, etc and have it approved before construction. It is important to remove the staking as soon as the tree is well-rooted as trunk strength can be weakened by long term staking.

H. Create a 6" saucer made of soil around the perimeter to hold water. This saucer should be just outside the transplanted root ball area.

Deep water transplanted tree on a regular basis and especially throughout dry periods to reduce water stress. Do not overwater especially where poor drainage and soil conditions occur. This could cause anaerobic environment around the root zone that is favorable to rot-causing fungi and bacteria, or may cause roots to

It is important when watering newly transplanted trees that the original soil ball and surrounding soil is saturated to a depth of 12". Apply water slowly to entire area, allowing adequate penetration. Watering is normally required weekly for the first month and twice a month for the rest of the first growing season. Watering should continue as needed for 2-3 seasons after transplanting. It is critical that evergreens be watered regularly and thoroughly until the soil freezes in order to prevent winter browning and needle desiccation due to dehydration by winter

J. After transplanting a tree, apply a 4-6" layer of elongated wood mulch, shredded bark or other organic mulch around the base of the tree, pulling it away from the trunk to reduce damage from trapped moisture. The mulched area should ideally reach out 10-12" past the original root ball. Trees with saucer around the edge of the mulch ring are ideal for retaining mulch through the first growing season. Remove saucer after the first season, creating a clean transition between mulch



#### TYPICAL PERENNIAL PLANTING DETAIL SCALE: 1/2"=1'-0"

NOTES:

- 1. FINISHED GRADE FOR SOD TO BE 1/2" BELOW TOP OF EDGING
- 2. TAMPER MULCH AT EDGING SO THAT IT DOES NOT SPILL INTO TURF.







TREE PROTECTION DETAIL SCALE: NTS



LOR FINISH / REMARKS	NOTE	DESCRIPTION	COLOR FINISH / REMARKS	NOTE	DESCRIPTION	COLOR FINISH / REMARKS	NOTE	
	(12)		COLOR: WHITE NO: LAREDO SERIES MANUFAC: HUBBELL	(18)	6' HIGH PVC FENCE	REFER TO SHEET C-1	24	Ν
IATCH OFFICE PARK ATCH OFFICE PARK	(13)	FIXED ALUM.	MANUFAC: SILVERLINE WINDOWS VINYL CLAD REFER TO SHEET A-12.2	(19)	FABRIC AWNING	COLOR: BLUE MANUFAC: HUDSON AWNING & SIGN CO. SEE REQUIRED VENDOR	25	Ν
	14	METAL FRAME DOORS	WITH GLASS	20		REFER TO SHEET C-2 SEE REQUIRED VENDOR		
K ALUMINUM GUTTER WITH LEAF	(15)	STOREFRONT ALUM FRAME DOORS	MANUFAC: KAWNEER REFER TO SHEET A-12.1	(21)		REFER TO SHEET A-14.1 SEE REQUIRED VENDOR		
TO UNDERGROUND DRAINAGE	(16)	METAL FRAME DOORS	REFER TO SHEET A-12.1	22	COLUMN	STL. COL. WRAPPED W/ SQUARE VINYL PVC TUBE NON-TAPERED ECONOMY PLAIN TO MATCH FENCE, REFER TO SHEET C-1 SEE REQUIRED AWNING VENDOR		
	(17)	4' HIGH FENCE	REFER TO SHEET C-1	23	EXTERIOR WATER FOUNTAIN	REFER TO SPECS		
	-						-	



COLOR FINISH / REMARKS	NOTE	DESCRIPTION	COLOR FINISH / REMARKS	NOTE	DESCRIPTION	COLOR FINISH / REMARKS	Ν
		EXTERIOR LIGHT	COLOR: WHITE NO: LAREDO SERIES MANUFAC: HUBBELL	18	6' HIGH PVC FENCE	REFER TO SHEET C-1	(
COLOR: MATCH OFFICE PARK STYLE: MATCH OFFICE PARK	(13)	FIXED ALUM.	MANUFAC: SILVERLINE WINDOWS VINYL CLAD REFER TO SHEET A-12.2	(19)	FABRIC AWNING	COLOR: BLUE MANUFAC: HUDSON AWNING & SIGN CO. SEE REQUIRED VENDOR	
	14	METAL FRAME DOORS	WITH GLASS	20		REFER TO SHEET C-2 SEE REQUIRED VENDOR	
6" TYPE K ALUMINUM GUTTER WITH LEAF SCREEN	(15)	STOREFRONT ALUM FRAME DOORS	MANUFAC: KAWNEER REFER TO SHEET A-12.1	21		REFER TO SHEET A-14.1 SEE REQUIRED VENDOR	
CONNECT TO UNDERGROUND DRAINAGE	(16)	METAL FRAME DOORS	REFER TO SHEET A-12.1	22	COLUMN	STL. COL. WRAPPED W/ SQUARE VINYL PVC TUBE NON-TAPERED ECONOMY PLAIN TO MATCH FENCE, REFER TO SHEET C-1 SEE REQUIRED AWNING VENDOR	
	(17)	4' HIGH FENCE	REFER TO SHEET C-1	(23)	EXTERIOR WATER FOUNTAIN	REFER TO SPECS	
				-			



CHURCH

SCALE: 1" = 20'

	GRID / TYPE	# PTS	SPAC	GROUP	AVE	MAX	MIN	MAX/MIN	AVE/M
Ft	Pavement / H-H	857	10.00	<+>	1.30	8.48	0.00	N/A	N/A
					·				
IIE									

LAMP         LUMENS         MOUNTING/BALLAST         LLF         QTY           (1) MHC70/U/M/4K         6200         WALL PACK WALL MOUNTED         0.72         14	LE					
		LAMP	LUMENS	MOUNTING/BALLAST	LLF	QTY
		(1) MHC70/U/M/4K	6200		0.72	14

						Where Great Places Begin	8000 South Lincoln Street #206   Littleton CO 80122	
		CHECKED MDC	APPROVED MDC	PROJECT NO. 12026	•	- HUKZ. SCALE 1" = 20'	VERT. SCALE n/a	
DATE NO. NOTES	07.12.2013 1 1ST SUBMITTAL	10.04.2013 2 2ND SUBMITTAL	01.31.2014 3 3RD SUBMITTAL					
S	HEE	TT		1	-	(	)	



December 12, 2013

Mark Cevaal, PE Redland 8000 South Lincoln Street, Suite 206 Littleton, CO 80122

#### **RE: The Learning Experience at Miramont Traffic Impact Study**

Dear Mr. Cevaal:

The Fox Tuttle Transportation Group has completed a traffic impact study for The Learning Experience (TLE) project proposed within the Miramont office park in Fort Collins. The project is proposing to construct a 10,000 square foot (SF) day care use on a currently vacant site located at the northwest corner of Boardwalk and Oakridge Drive. Access is proposed at existing access locations along Boardwalk aligning with Oakridge Drive and with a Sam's Club access driveway.

The purpose of this study is to assist in identifying potential traffic impacts within the study area as a result of this development project. The traffic study addresses existing and near-term (Year 2015) peak hour intersection conditions in the study area. The information contained in this study is anticipated to be used by the City in identifying any intersection or roadway deficiencies and potential improvements that may be required of the project. This memorandum summarizes our analyses, findings, and recommendations.

#### **Project Description**

The project proposes to develop a 10,000 SF day care facility. A vicinity map is shown on **Figure 1**. The proposed site and access plan is provided on **Figure 2**.

Access to the site is proposed as follows:

- Access on Boardwalk via the existing west leg of the Boardwalk & Oakridge Drive intersection
- Access on Boardwalk via an existing shared driveway aligning with the Sam's Club access approximately 230' north of Oakridge Drive

Both accesses are shared with existing office use in the Miramont development.

#### **Study Area**

The study area boundaries were developed in consultation with City staff and took into consideration the amount of site traffic added to the surrounding street network and planned access. The existing study area street network consists of arterial and collector streets. The primary public roadways that serve the project site are discussed in the following text.

*E. Harmony Road* is a four-lane to six-lane major arterial with bicycle lanes that provides eastwest access through the City of Fort Collins. The posted speed limit on Harmony Road is 45 miles per hour (mph) in the site vicinity.

**Boardwalk** is a two-lane collector roadway with bicycle lanes that provides north-south access through the study area with direct access to adjacent uses. The posted speed limit on Boardwalk is 30 mph in the site vicinity. The intersection of Boardwalk with E. Harmony Road is controlled with a traffic signal.

**Oakridge Drive** is a two-lane collector roadway that provides east-west access through the immediate area with direct access to adjacent uses. The posted speed limit on Oakridge Drive is 25 mph in the site vicinity. The intersection of Oakridge with Boardwalk is controlled with stop signs on the minor street (Oakridge) approaches.

#### **Existing Traffic Volumes**

Weekday AM / PM peak hour turning-movement and daily roadway volumes were collected in July and September 2013 for this project. The existing traffic volumes are illustrated on **Figure 3**. Count data sheets are provided in the Appendix.

#### **Existing Intersection Capacity and Queue Analysis**

In determining the operational characteristics of an intersection, "Levels of Service" (LOS) A through F are applied, with LOS A indicating very good operations and LOS F indicating congested operations. The intersection LOS is represented as a delay in seconds per vehicle for the intersection as a whole and for each turning movement. A more detailed discussion of LOS methodology is contained in the Appendix for reference. Criteria contained in the Highway Capacity Manual (HCM) was applied for these analyses in order to determine existing levels of service during peak hour periods.

The results of the LOS calculations for the intersections are summarized in **Table 1**. The intersection level of service worksheets are attached in the Appendix. The data in the tables show that all study area intersections are operating with acceptable overall levels of service. No existing capacity deficiencies or mitigation measures were identified for existing traffic volumes.

#### **Future Traffic Volumes and Roadway Network**

Per discussions with City staff, a 1.25% annual growth rate was assumed to account for future background traffic growth in the study area. There are no major roadway network or capacity improvements planned by the City within the study area within the short-term planning horizon. Using these assumptions, the Year 2015 background traffic volumes were calculated and are summarized on **Figure 4**.

#### Year 2015 Background Scenario Analysis (Without Proposed Development)

The study area intersections were evaluated to determine baseline operations for the 2015 scenario and to identify any capacity constraints associated with background traffic. The Level of Service criteria discussed in prior sections was applied to the study area intersections to determine impacts with the addition of site build out traffic volumes in the short-term. The results of the LOS calculations for the intersections are summarized in **Table 1**. The intersection level of service worksheets are attached in the Appendix.

The data **Table 1** shows that all study area intersections will continue to operate well overall with no changes in overall intersection or movement Levels of Service. Therefore, no capacity deficiencies or mitigation measures were identified for the Year 2015 background traffic scenario.

#### **Trip Generation**

To establish the volume of new trips that will be added to the area roadway network with redevelopment of the site, trip generation estimates for the proposed site uses were calculated based on rates contained in the Institute of Transportation Engineers (ITE) trip generation manual. The ITE trip rates for land use #565 "Day Care Center" were applied to estimate proposed traffic for the site.

As shown in **Table 2** and based on ITE methodology and the assumptions discussed in this section, the project is anticipated to generate the following trips at build out:

- 741 weekday daily trips
- 122 weekday AM peak hour trips
- 123 weekday PM peak hour trips

#### **Trip Distribution and Assignment**

The estimated traffic volumes presented in **Table 2** was distributed onto the adjacent street network based on existing traffic characteristics of the area, as well as land use and traffic patterns in the greater project area.

Using these distribution assumptions, the projected site traffic was assigned to the study area roadway network for the weekday AM and weekday PM peak hour periods. The site-generated volumes are shown on **Figure 5** along with the assumed distribution percentages.

#### Intersection Capacity Analysis for Year 2015 + Project Scenario

The site-generated traffic volumes were added to the Year 2015 background volumes to analyze potential site impacts in the short-term build out scenario. The Year 2015 + site-generated traffic volumes are illustrated on **Figure 6**. The level of service criteria discussed in prior sections was applied to the study area intersections to determine impacts with the addition of site-build out traffic volumes in the short-term. The results of the LOS calculations for the intersections are summarized in **Table 1**.

The data contained in **Table 1** illustrates that all study area intersections and individual movements will continue to operate acceptably overall (LOS E or better).

No deficiencies or mitigation measures were identified. The LOS analysis shows that the existing northbound shared left-through-right lane on Boardwalk at Oakridge can continue to service volumes with the project with minimal delays. Given the 30 mph speed limit on Boardwalk, the additional right-turn volumes at Sam's Club and Oakridge Drive accesses do not warrant the addition of right-turn deceleration lanes at these accesses using NCHRP Report 273 criteria. The LOS result also do not indicate capacity constraints with the existing shared through-right lane configurations.

#### Circulation and Drop-Off/Pick-Up

The TLE will operate from 6:30am to 6:30pm, Monday through Friday. The TLE will offer child care to children ages six weeks to five years, with after-school care for children up to eight years of age. Parents will drop children off throughout the morning with no set "bell" time or concentrated arrival times, unlike an elementary or typical public school. Similarly, parents will pick up children throughout the

afternoon and evening hours, with no set bell time and arrivals and departures staggered throughout the peak hours. Drop-off and pick-up activities will occur at random over the AM and PM periods and at the convenience of the parents. Parents are required to park, escort, and check-in/check-out all children into and out of the facility and students will never be dropped off or picked-up outside the facility unattended. Per TLE data, drop-off and pick-up of children will typically take between 5-8 minutes. Based on these characteristics, there is no vehicle queuing or waiting that will occur that may be associated with traditional (set bell time) schools at drop-off and pick-up.

The following data was provided by the applicant for a similar TLE site with a 183-child capacity and illustrates the spread of drop-off and pick-up activity throughout the AM and PM periods:

- Traffic during drop-off (average 5 minutes):
  - o 6:30am-7:00am (11 children)
  - o 7:00am-7:30am (28 children)
  - o 7:30am-8:00am (43 children)
  - o 8:00am-8:30am (43 children)
  - o 8:30am-9:00am (29 children)
  - o 9:00am-9:30am (29 children)
- Traffic during pick-up (average 8 minutes):
  - o 3:30pm-4:00pm (11 children)
  - o 4:00pm-4:30pm (28 children)
  - o 4:30pm-5:00pm (43 children)
  - o 5:00pm-5:30pm (43 children)
  - o 5:30pm-6:00pm (29 children)
  - o 6:00pm-6:30pm (29 children)

The above data suggests that, on average, there may be three to five parents parked to drop off at any time during the AM peak and five to six parents at any time during the PM peak. Given the random arrivals and departures and the requirement that parents must park and walk into and out of the facility with their children, circulation or queuing issues are not anticipated. The existing parking lot will easily accommodate these activities. TLE staff should be encouraged to park furthest away from the front door so as to minimize the distance parents and children will need to walk.

#### Conclusions

The Learning Experience at Miramont project is proposing to construct a 10,000 SF day care facility at the northwest corner of Boardwalk and Oakridge Drive in the City of Fort Collins. Access is proposed at existing access locations along Boardwalk.

This traffic study evaluated existing and short term (Year 2015) peak hour intersection conditions in the study area with the project to identify potential operational issues and to recommend mitigation measures.

The project is anticipated to generate approximately 741 daily trips, with 122 trips occurring in the AM peak hour and 123 trips occurring in the PM peak hour. It was determined that the project-added traffic volumes can be accommodated on the existing roadway and intersection network with minimal effects. No mitigation measures were identified as necessary to support development of the project as proposed.

Sincerely, FOX TUTTLE TRANSPORTATION GROUP, LLC

Steve Jutth

Steve Tuttle, P.E., PTOE Principal



Tables and Figures:

- Table 1 Peak Hour Intersection LOS Summary
- Table 2 Trip Generation Estimate
- Figure 1 Site Vicinity
- Figure 2 Site Plan
- Figure 3 Existing Traffic Volumes
- Figure 4 Year 2015 Background Traffic Volumes
- Figure 5 Site-Generated Traffic Volumes
- Figure 6 Year 2015 + Site-Generated Traffic Volumes

#### The Learning Experience at Miramont Traffic Impact Study



#### Table 1 - Intersection Level of Service Summary

		Exis	sting		Ye	ear 2015 I	Backgrou	nd	Y	ear 2015	w/ Projec	t
Intersection and	AM I	Peak	PM	Peak	AM I	Peak	PM	Peak	AM I	Peak	PM I	Peak
Lanes Groups	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SIGNAL CONTROL												
Harmony Rd & Boardwalk	17.0	В	30.9	С	17.2	В	31.9	С	17.9	В	33.8	С
Eastbound Left	7.3	A	18.3	В	7.5	Α	19.2	В	7.6	А	20.6	С
Eastbound Through	11.8	В	26.7	С	12.0	В	27.8	С	12.3	В	26.5	С
Eastbound Right	9.0	Α	20.2	С	9.1	Α	20.7	С	9.6	А	21.1	С
Westbound Left	8.3	Α	27.5	С	8.4	Α	30.4	С	8.8	Α	30.3	С
Westbound Through	10.5	В	22.8	С	10.7	В	23.6	С	10.8	В	24.7	С
Westbound Right	10.4	В	18.3	В	10.6	В	18.8	В	10.7	В	18.2	В
Northbound Left	37.9	D	47.4	D	37.9	D	50.1	D	37.6	D	77.8	Е
Northbound Through	49.9	D	52.8	D	50.3	D	53.0	D	51.3	D	56.0	Е
Northbound Right	44.5	D	47.9	D	44.5	D	48.0	D	45.7	D	49.7	D
Southbound Left	36.6	D	48.8	D	37.6	D	51.7	D	39.0	D	50.1	D
Southbound Through	43.7	D	67.3	Е	44.0	D	68.8	Е	46.1	D	67.8	Е
Southbound Right	44.5	D	47.2	D	45.2	D	47.2	D	47.2	D	47.2	D
STOP CONTROL												
Boardwalk & Oakridge Dr	3.6	Α	3.4	Α	3.7	Α	3.4	Α	4.3	Α	4.2	Α
Eastbound Left+Through+Right	10.3	В	12.9	В	10.4	В	12.8	В	12.4	В	15.8	С
Westbound Left	11.3	В	14.1	В	11.4	В	14.5	В	11.8	В	15.3	С
Westbound Through+Right	10.0	Α	10.2	В	10.1	В	10.3	В	10.4	В	10.6	В
Northbound Left	0.2	Α	0.0	Α	0.2	Α	0.0	Α	0.6	Α	0.5	Α
Southbound Left	7.9	Α	7.9	Α	7.9	Α	8.0	А	7.9	А	8.0	А
Boardwalk & Sam's Club	0.5	Α	1.8	Α	0.6	Α	1.9	Α	1.0	Α	2.2	Α
Eastbound Left+Through+Right	10.9	В	13.4	В	11.1	В	13.7	В	11.8	В	15.4	С
Westbound Left+Through+Right	9.9	Α	11.3	В	10.0	Α	11.6	В	10.2	В	11.9	В
Northbound Left	7.7	Α	8.2	Α	7.7	Α	8.3	Α	7.8	Α	8.4	А
Southbound Left	7.8	А	8.0	А	7.9	A	8.0	А	7.9	А	8.1	А

Note: Delay represented in average seconds per vehicle.

#### FT#13068

#### The Learning Experience at Miramont Traffic Impact Study



#### Table 2. Trip Generation Estimate

				Average Daily Trips			A.M. Peak Hour Trips				P.M. Peak Hour Trips			
Land Use	Size	Unit	Rate	Total	In	Out	Rate	Total	In	Out	Rate	Total	In	Out
Day Care Center - ITE #565	10	1,000 SF	74.06	741	371	370	12.18	122	65	57	12.34	123	58	65

Source: ITE Trip Generation 9th Edition. 2012.












### APPENDIX

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Attachment A – TIS Base Assumptions Level of Service Definitions Intersection Capacity Worksheets Traffic Count Data Sheets

Attachment A – TIS Base Assumptions

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### Attachment A Transportation Impact Study Base Assumptions

Project Information		
Project Name The Cearming Project Location P.	Emana & Duramon	+ DEFICE Part ADA
Project Location Boardwalk	+ Oakalae	
TIS Assumptions		
Type of Study	Full:	Intermediate:
Study Area Boundaries	North: Harmony Rd	South: Oskadge
	East: Boundary	West:
Study Years	Short Range: 20/5	Long Range:
Future Traffic Growth Rate	1.25%	
Study Intersections	1. All access drives5.	
	2. Beardwell + Harmon 6.	
	3. BOArdwAk, Onkridge 7.	
	4. 8.	
Time Period for Study	AM: 7:00-9:00 PM: 4:00-6:00	Sat Noon:
Trip Generation Rates	ITE	
Trip Adjustment Factors		aptive farket:
Overall Trip Distribution	SEE ATTACHED	SKETCH
Mode Split Assumptions	NONE	
Committed Roadway Improvements	NONE	
Other Traffic Studies	NONE	
Areas Requiring Special Study	- Borrdwalk: SbRturnlou HbLturn Come @ Site A Discuss + Analyze drog	
Date: <u>8/28/13</u>	Site tRACTIC CIrculation tRAFFIC from backing	plans to keep up on to Beardwalk
Fraffic Engineer	Ĩ	

Traffic Engineer:

Local Entity Engineer:

8/28/13

Level of Service Definitions

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### **LEVEL OF SERVICE DEFINITIONS**

In rating roadway and intersection operating conditions with existing or future traffic volumes, "Levels of Service" (LOS) A through F are used, with LOS A indicating very good operation and LOS F indicating poor operation. Levels of service at signalized and unsignalized intersections are closely associated with vehicle delays experienced in seconds per vehicle. More complete level of service definitions and delay data for signal and stop sign controlled intersections are contained in the following table for reference.

Level	Delay in seco	onds per vehicle <i>(a)</i>	
of Service Rating	Signalized	Unsignalized	Definition
А	0.0 to 10.0	0.0 to 10.0	Low vehicular traffic volumes; primarily free flow operations. Density is low and vehicles can freely maneuver within the traffic stream. Drivers are able to maintain their desired speeds with little or no delay.
В	10.1 to 20.0	10.1 to 15.0	Stable vehicular traffic volume flow with potential for some restriction of operating speeds due to traffic conditions. Vehicle maneuvering is only slightly restricted. The stopped delays are not bothersome and drivers are not subject to appreciable tension.
с	20.1 to 35.0	15.1 to 25.0	Stable traffic operations, however the ability for vehicles to maneuver is more restricted by the increase in traffic volumes. Relatively satisfactory operating speeds prevail, but adverse signal coordination or longer vehicle queues cause delays along the corridor.
D	35.1 to 55.0	25.1 to 35.0	Approaching unstable vehicular traffic flow where small increases in volume could cause substantial delays. Most drivers are restricted in ability to maneuver and selection of travel speeds due to congestion. Driver comfort and convenience are low, but tolerable.
E	55.1 to 80.0	35.1 to 50.0	Traffic operations characterized by significant approach delays and average travel speeds of one-half to one-third the free flow speed. Vehicular flow is unstable and there is potential for stoppages of brief duration. High signal density, extensive vehicle queuing, or corridor signal progression/timing are the typical causes of vehicle delays at signalized corridors.
F	> 80.0	> 50.0	Forced vehicular traffic flow and operations with high approach delays at critical intersections. Vehicle speeds are reduced substantially and stoppages may occur for short or long periods of time because of downstream congestion.

(a) Delay ranges based on 2010 Highway Capacity Manual criteria.

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Intersection Capacity Worksheets

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	4			4		ሻ	eî 👘	
Volume (veh/h)	0	1	1	18	3	79	4	167	23	82	93	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	1	1	21	4	93	5	196	27	96	109	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	618	536	111	524	524	210	112			224		
vC1, stage 1 conf vol	304	304		219	219							
vC2, stage 2 conf vol	314	233		304	305							
vCu, unblocked vol	618	536	111	524	524	210	112			224		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	96	99	89	100			93		
cM capacity (veh/h)	474	536	943	593	560	830	1478			1345		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	2	21	96	228	96	112						
Volume Left	0	21	0	5	96	0						
Volume Right	1	0	93	27	0	2						
cSH	683	593	816	1478	1345	1700						
Volume to Capacity	0.00	0.04	0.12	0.00	0.07	0.07						
Queue Length 95th (ft)	0	3	10	0	6	0						
Control Delay (s)	10.3	11.3	10.0	0.2	7.9	0.0						
Lane LOS	В	В	В	A	A							
Approach Delay (s)	10.3	10.2	2	0.2	3.7							
Approach LOS	B	B		0.2	0.7							
Intersection Summary												
Average Delay			3.6									
Intersection Capacity Utilization	on		33.1%	IC	Ulevelo	of Service			А			
Analysis Period (min)			15		0 201010							
			10									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		- ሽ	eî 🗧		ሻ	4î	
Volume (veh/h)	4	0	0	1	0	10	3	240	3	6	176	13
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	5	0	0	1	0	12	4	282	4	7	207	15
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (ft)											1102	
pX, platoon unblocked												
vC, conflicting volume	530	522	215	512	528	284	222			286		
vC1, stage 1 conf vol	229	229		291	291							
vC2, stage 2 conf vol	301	293		221	236							
vCu, unblocked vol	530	522	215	512	528	284	222			286		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	100	100	100	100	98	100			99		
cM capacity (veh/h)	616	593	825	634	593	755	1347			1276		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	5	13	4	286	7	222						
Volume Left	5	1	4	0	7	0						
Volume Right	0	12	0	4	0	15						
cSH	616	742	1347	1700	1276	1700						
Volume to Capacity	0.01	0.02	0.00	0.17	0.01	0.13						
Queue Length 95th (ft)	1	1	0	0	0	0						
Control Delay (s)	10.9	9.9	7.7	0.0	7.8	0.0						
Lane LOS	В	Α	А		А							
Approach Delay (s)	10.9	9.9	0.1		0.2							
Approach LOS	В	А										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization	on		22.8%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>††</b>	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1
Volume (veh/h)	51	901	123	49	736	248	106	109	39	125	69	65
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	0.99		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	422	2257	996	351	2256	995	300	175	147	283	204	172
Arrive On Green	0.04	0.61	0.61	0.04	0.61	0.61	0.08	0.09	0.09	0.10	0.11	0.11
Sat Flow, veh/h	1845	3725	1644	1774	3725	1644	1774	1863	1563	1774	1863	1566
Grp Volume(v), veh/h	60	1060	145	58	775	292	112	118	46	147	81	76
Grp Sat Flow(s),veh/h/ln	1845	1863	1644	1774	1863	1644	1774	1863	1563	1774	1863	1566
Q Serve(g_s), s	1.2	16.0	3.9	1.2	10.6	8.7	5.7	6.3	2.8	7.3	4.1	4.7
Cycle Q Clear(g_c), s	1.2	16.0	3.9	1.2	10.6	8.7	5.7	6.3	2.8	7.3	4.1	4.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	422	2257	996	351	2256	995	300	175	147	283	204	172
V/C Ratio(X)	0.14	0.47	0.15	0.17	0.34	0.29	0.37	0.67	0.31	0.52	0.40	0.44
Avail Cap(c_a), veh/h	507	2257	996	434	2256	995	327	227	191	283	227	191
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	7.2	11.1	8.7	8.0	10.1	9.7	37.1	44.9	43.3	35.0	42.4	42.7
Incr Delay (d2), s/veh	0.2	0.7	0.3	0.2	0.4	0.7	0.8	5.1	1.2	1.7	1.2	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/In	0.5	6.7	1.4	0.5	4.4	3.2	2.6	3.2	1.1	3.4	2.1	1.9
Lane Grp Delay (d), s/veh	7.3	11.8	9.0	8.3	10.5	10.4	37.9	49.9	44.5	36.6	43.7	44.5
Lane Grp LOS	A	B	Α	A	B	В	D	D	D	D	D	D
Approach Vol, veh/h		1265			1125			276			304	
Approach Delay, s/veh		11.3			10.4			44.2			40.5	_
Approach LOS		В			В			D			D	
Timer												
Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.3	67.0		7.2	67.0		11.4	15.1		13.0	16.7	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	8.0	61.0		8.0	61.0		9.0	11.5		9.0	11.5	
Max Q Clear Time (g_c+l1), s	3.2	18.0		3.2	12.6		7.7	8.3		9.3	6.7	
Green Ext Time (p_c), s	0.0	12.9		0.0	13.1		0.0	0.3		0.0	0.5	
Intersection Summary												
HCM 2010 Ctrl Delay			17.0									
HCM 2010 LOS			В									
Notes												

TLE at Miramont Traffic Impact Study
Fox Tuttle Transportation Group, LLC

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$		1	et			÷		ľ	et.	
Volume (veh/h)	2	3	3	25	0	118	0	166	26	103	288	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	2	4	4	29	0	139	0	195	31	121	339	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	931	807	339	797	792	211	339			226		
vC1, stage 1 conf vol	581	581		211	211							
vC2, stage 2 conf vol	349	226		586	581							
vCu, unblocked vol	931	807	339	797	792	211	339			226		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	99	99	93	100	83	100			91		
cM capacity (veh/h)	351	417	703	423	429	830	1220			1343		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	9	29	139	226	121	339						
Volume Left	2	29	0	0	121	0						
Volume Right	4	0	139	31	0	0						
cSH	467	423	830	1220	1343	1700						
Volume to Capacity	0.02	0.07	0.17	0.00	0.09	0.20						
Queue Length 95th (ft)	2	6	15	0	7	0						
Control Delay (s)	12.9	14.1	10.2	0.0	7.9	0.0						
Lane LOS	В	В	В		А							
Approach Delay (s)	12.9	10.9		0.0	2.1							
Approach LOS	В	В										
Intersection Summary												
Average Delay			3.4									
Intersection Capacity Utilizat	ion		42.8%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									
<u> </u>												

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EBL

Movement

LDL	LDI	LDI	WDL	VVDI	VVDI		NDT	NDI		301	301
	- ↔			4			eî 👘			eî 👘	
23	1	9	11	0	47	5		5	29		-
	Stop						Free			Free	
	0%			0%						0%	
0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
27	1	11	13	0	55	6	325	6	34	436	
							TWLTL			TWLTL	
							2			2	
										1102	
897	848	437	855	845	328	438			331		
505	505		339	339							
392	342		516	506							
897	848	437	855	845	328	438			331		
7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
6.1	5.5		6.1	5.5							
3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
94	100	98	97	100	92	99			97		
428	456	619	455	460	714	1122			1229		
EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
39	68	6	331	34	438						
		6									
11		0	6	0	1						
					1700						
7											
13.4	11.3	8.2	0.0	8.0	0.0						
В	В										
		1.8									
		36.2%	10		of Service			А			
า		30.2%	IU IU	O LEVELU				A			
l		36.2% 15	IC					A			
	27 897 505 392 897 7.1 6.1 3.5 94 428 <b>EB1</b> 39 428 <b>EB1</b> 39 27 11 468 0.08 7 13.4 B 13.4	23       1         23       1         Stop       0%         0.85       0.85         27       1         27       1         27       1         897       848         505       505         392       342         897       848         7.1       6.5         392       342         897       848         7.1       6.5         3.5       4.0         94       100         428       456         EB 1       WB 1         39       68         27       13         11       55         468       644         0.08       0.11         7       9         13.4       11.3         B       8         13.4       11.3	23       1       9         Stop       0%         0.85       0.85       0.85         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         27       1       11         2897       848       437         505       505       392         392       342       437         7.1       6.5       6.2         6.1       5.5       3.3         94       100       98         428       456       619         EB1       WB1       NB1         39       68       6         27       13       6         27       13       6         20.8       0.11       0.01         7       9       0         13.4       11.3       8.2         8       8       4 <td>Partial       Partial       Partial         23       1       9       11         Stop       0%           0.85       0.85       0.85       0.85       0.85         27       1       11       13         27       1       11       13         27       1       11       13         27       1       11       13         897       848       437       855         505       505       339         392       342       516         897       848       437       855         7.1       6.5       6.2       7.1         6.1       5.5       6.1       3.5         94       100       98       97         428       456       619       455         50       61       3.31       3.5         94       100       98       97         428       456       619       455         50       6       6       331         27       13       6       0         134       155       0       6</td> <td>Partial       Partial       Partial         23       1       9       11       0         Stop       0%       0%       0%         0.85       0.85       0.85       0.85       0.85         27       1       11       13       0         277       1       11       13       0         897       848       437       855       845         505       505       339       339         392       342       516       506         897       848       437       855       845         505       505       505       339       339         392       342       516       506         897       848       437       855       845         7.1       6.5       6.2       7.1       6.5         6.1       5.5       6.1       5.5       3.5       4.0         3.5       4.0       3.3       3.5       4.0         94       100       98       97       100         428       456       619       455       360         3.5       0       6       0       &lt;</td> <td>4911047StopStop0%0%0%0.850.850.850.850.850.850.850.850.850.852711113055271111305527111130553711113055393923425165068978484378558453287.16.56.27.16.56.26.15.56.15.55.53.54.03.33.54.03.394100989710092428456619455460714EB 1WB 1NB 1NB 2SB 1SB 239686331344382713603401155060146864411221700122917000.080.110.010.190.030.2679002013.411.30.10.6BBAA13.411.30.10.6</td> <td>23         1         9         11         0         47         5           Stop         Stop         0%         0%         0%         0%           0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85           27         1         11         13         0         55         6           27         1         11         13         0         55         6           27         1         11         13         0         55         6           28         7         1         11         13         0         55         6           297         848         437         855         845         328         438           505         505         339         339         328         438           7.1         6.5         6.2         7.1         6.5         6.2         4.1           6.1         5.5         6.1         5.5         5         3.3         2.2         9           94         100         98         97         100         92         99           428         456         619         455</td> <td>A         A         Y         F           23         1         9         11         0         47         5         276           Stop         Stop         Stop         Free         0%         0%         0%           0.85         0</td> <td><math display="block">\begin{array}{c ccccccccccccccccccccccccccccccccccc</math></td> <td><math display="block">\begin{array}{c ccccccccccccccccccccccccccccccccccc</math></td> <td><math display="block"> \begin{array}{c ccccccccccccccccccccccccccccccccccc</math></td>	Partial       Partial       Partial         23       1       9       11         Stop       0%           0.85       0.85       0.85       0.85       0.85         27       1       11       13         27       1       11       13         27       1       11       13         27       1       11       13         897       848       437       855         505       505       339         392       342       516         897       848       437       855         7.1       6.5       6.2       7.1         6.1       5.5       6.1       3.5         94       100       98       97         428       456       619       455         50       61       3.31       3.5         94       100       98       97         428       456       619       455         50       6       6       331         27       13       6       0         134       155       0       6	Partial       Partial       Partial         23       1       9       11       0         Stop       0%       0%       0%         0.85       0.85       0.85       0.85       0.85         27       1       11       13       0         277       1       11       13       0         897       848       437       855       845         505       505       339       339         392       342       516       506         897       848       437       855       845         505       505       505       339       339         392       342       516       506         897       848       437       855       845         7.1       6.5       6.2       7.1       6.5         6.1       5.5       6.1       5.5       3.5       4.0         3.5       4.0       3.3       3.5       4.0         94       100       98       97       100         428       456       619       455       360         3.5       0       6       0       <	4911047StopStop0%0%0%0.850.850.850.850.850.850.850.850.850.852711113055271111305527111130553711113055393923425165068978484378558453287.16.56.27.16.56.26.15.56.15.55.53.54.03.33.54.03.394100989710092428456619455460714EB 1WB 1NB 1NB 2SB 1SB 239686331344382713603401155060146864411221700122917000.080.110.010.190.030.2679002013.411.30.10.6BBAA13.411.30.10.6	23         1         9         11         0         47         5           Stop         Stop         0%         0%         0%         0%           0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85           27         1         11         13         0         55         6           27         1         11         13         0         55         6           27         1         11         13         0         55         6           28         7         1         11         13         0         55         6           297         848         437         855         845         328         438           505         505         339         339         328         438           7.1         6.5         6.2         7.1         6.5         6.2         4.1           6.1         5.5         6.1         5.5         5         3.3         2.2         9           94         100         98         97         100         92         99           428         456         619         455	A         A         Y         F           23         1         9         11         0         47         5         276           Stop         Stop         Stop         Free         0%         0%         0%           0.85         0	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>††</b>	1	٦	<u></u>	1	٦	<b>•</b>	1	٦	•	7
Volume (veh/h)	60	1244	317	110	1274	297	258	173	104	258	201	94
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	213	1893	835	210	1956	863	345	281	237	376	281	237
Arrive On Green	0.04	0.51	0.51	0.06	0.52	0.52	0.14	0.15	0.15	0.14	0.15	0.15
Sat Flow, veh/h	1845	3725	1643	1774	3725	1644	1774	1863	1571	1774	1863	1571
Grp Volume(v), veh/h	71	1464	373	129	1341	349	272	188	122	304	236	111
Grp Sat Flow(s),veh/h/ln	1845	1863	1643	1774	1863	1644	1774	1863	1571	1774	1863	1571
Q Serve(g_s), s	2.1	37.6	17.0	3.8	31.5	15.1	15.1	11.3	8.4	17.0	14.5	7.6
Cycle Q Clear(g_c), s	2.1	37.6	17.0	3.8	31.5	15.1	15.1	11.3	8.4	17.0	14.5	7.6
Prop In Lane	1.00	1000	1.00	1.00	1057	1.00	1.00	201	1.00	1.00	201	1.00
Lane Grp Cap(c), veh/h	213 0.33	1893 0.77	835 0.45	210 0.61	1956	863	345 0.79	281 0.67	237 0.51	376 0.81	281	237 0.47
V/C Ratio(X) Avail Cap(c_a), veh/h	264	1893	0.45 835	229	0.69 1956	0.40 863	345	292	246	376	0.84 292	246
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.4	23.5	18.5	23.3	20.8	16.9	35.8	47.3	46.1	36.3	48.7	45.8
Incr Delay (d2), s/veh	0.9	3.1	10.5	4.2	20.0	10.9	11.6	5.5	1.7	12.5	18.6	45.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/ln	0.9	17.5	6.9	2.2	14.3	6.1	8.5	5.9	3.4	9.1	8.5	3.1
Lane Grp Delay (d), s/veh	18.3	26.7	20.2	27.5	22.8	18.3	47.4	52.8	47.9	48.8	67.3	47.2
Lane Grp LOS	В	С	C	С	C	В	D	D	D	D	E	D
Approach Vol, veh/h		1908			1819			582			651	
Approach Delay, s/veh		25.1			22.3			49.3			55.2	
Approach LOS		С			С			D			E	
Timer												
Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.8	65.0		9.7	67.0		20.0	23.3		20.0	23.3	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	7.0	59.0		7.0	59.0		16.0	17.5		16.0	17.5	
Max Q Clear Time (q_c+I1), s		39.6		5.8	33.5		17.1	13.3		19.0	16.5	
Green Ext Time (p_c), s	0.0	15.3		0.0	18.8		0.0	1.0		0.0	0.3	
Intersection Summary												
HCM 2010 Ctrl Delay			30.9									
HCM 2010 LOS			С									
Notes												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	4			4		ሻ	ef 👘	
Volume (veh/h)	0	1	1	20	5	80	5	170	25	85	95	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	1	1	24	6	94	6	200	29	100	112	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	636	554	113	540	541	215	114			229		
vC1, stage 1 conf vol	313	313		226	226							
vC2, stage 2 conf vol	324	241		314	314							
vCu, unblocked vol	636	554	113	540	541	215	114			229		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	96	99	89	100			93		
cM capacity (veh/h)	461	526	940	583	551	825	1475			1339		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	2	24	100	235	100	114						
Volume Left	0	24	0	6	100	0						
Volume Right	1	0	94	29	0	2						
cSH	675	583	802	1475	1339	1700						
Volume to Capacity	0.00	0.04	0.12	0.00	0.07	0.07						
Queue Length 95th (ft)	0	3	11	0	6	0						
Control Delay (s)	10.4	11.4	10.1	0.2	7.9	0.0						
Lane LOS	В	В	В	А	А							
Approach Delay (s)	10.4	10.4		0.2	3.7							
Approach LOS	В	В										
Intersection Summary												
Average Delay			3.7									
Intersection Capacity Utiliza	tion		33.6%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		<u>۲</u>	eî 🗧		ሻ	4î	
Volume (veh/h)	5	0	0	1	0	10	5	245	5	10	180	15
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	6	0	0	1	0	12	6	288	6	12	212	18
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (ft)											1102	
pX, platoon unblocked												
vC, conflicting volume	556	550	221	538	556	291	229			294		
vC1, stage 1 conf vol	244	244		303	303							
vC2, stage 2 conf vol	312	306		235	253							
vCu, unblocked vol	556	550	221	538	556	291	229			294		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	100	100	100	100	98	100			99		
cM capacity (veh/h)	599	577	819	619	579	748	1339			1267		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	6	13	6	294	12	229						
Volume Left	6	1	6	0	12	0						
Volume Right	0	12	0	6	0	18						
cSH	599	734	1339	1700	1267	1700						
Volume to Capacity	0.01	0.02	0.00	0.17	0.01	0.13						
Queue Length 95th (ft)	1	1	0	0	1	0						
Control Delay (s)	11.1	10.0	7.7	0.0	7.9	0.0						
Lane LOS	В	A	A	010	A	010						
Approach Delay (s)	11.1	10.0	0.2		0.4							
Approach LOS	В	A	0.2		0							
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ition		23.2%	IC	CU Level (	of Service			А			
Analysis Period (min)			15									
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	<u>††</u>	1	ľ	<u></u>	1	ľ	•	1	ľ	•	1
Volume (veh/h)	55	925	125	50	755	255	110	110	40	130	70	70
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	0.99		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	414	2255	995	342	2251	993	300	177	148	282	202	170
Arrive On Green	0.04	0.61	0.61	0.04	0.60	0.60	0.08	0.09	0.09	0.10	0.11	0.11
Sat Flow, veh/h	1845	3725	1644	1774	3725	1644	1774	1863	1563	1774	1863	1566
Grp Volume(v), veh/h	65	1088	147	59	795	300	116	120	47	153	82	82
Grp Sat Flow(s),veh/h/ln	1845	1863	1644	1774	1863	1644	1774	1863	1563	1774	1863	1566
Q Serve(g_s), s	1.3	16.7	4.0	1.3	11.0	9.1	5.9	6.4	2.9	7.7	4.2	5.1
Cycle Q Clear(g_c), s	1.3	16.7	4.0	1.3	11.0	9.1	5.9	6.4	2.9	7.7	4.2	5.1
Prop In Lane	1.00	2255	1.00	1.00	2251	1.00	1.00	177	1.00	1.00	202	1.00
Lane Grp Cap(c), veh/h	414	2255	995 0.15	342 0.17	2251	993	300	177	148	282	202 0.41	170
V/C Ratio(X) Avail Cap(c_a), veh/h	0.16 497	0.48 2255	0.15 995	424	0.35 2251	0.30 993	0.39 324	0.68 227	0.32 190	0.54 282	227	0.48 191
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	7.3	11.3	8.8	8.2	10.2	9.8	37.0	44.9	43.3	35.5	42.7	43.1
Incr Delay (d2), s/veh	0.2	0.7	0.0	0.2	0.4	0.8	0.8	5.4	1.2	2.1	1.3	2.1
Initial Q Delay(d3), s/veh	0.2	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/ln	0.5	7.0	1.4	0.5	4.6	3.3	2.7	3.3	1.2	3.6	2.1	2.1
Lane Grp Delay (d), s/veh	7.5	12.0	9.1	8.4	10.7	10.6	37.9	50.3	44.5	37.6	44.0	45.2
Lane Grp LOS	A	В	A	A	В	В	D	D	D	D	D	D
Approach Vol, veh/h		1300			1154			283			317	
Approach Delay, s/veh		11.5			10.5			44.3			41.2	
Approach LOS		В			В			D			D	
Timer		_			_			_				
Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.4	67.1		7.3	67.0		11.6	15.2		13.0	16.6	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	8.0	61.0		4.0 8.0	61.0		9.0	11.5		9.0	11.5	
Max Q Clear Time $(q_c+11)$ , s	3.3	18.7		3.3	13.0		7.9	8.4		9.7	7.1	
Green Ext Time (p_c), s	0.0	13.4		0.0	13.7		0.0	0.3		0.0	0.5	
Intersection Summary												
HCM 2010 Ctrl Delay			17.2									
HCM 2010 LOS			B									
Notes												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$		ሻ	et			\$		ሻ	ef 👘	
Volume (veh/h)	2	5	5	25	0	120	0	170	25	105	295	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	2	6	6	29	0	141	0	200	29	124	347	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	950	824	347	818	809	215	347			229		
vC1, stage 1 conf vol	594	594		215	215							
vC2, stage 2 conf vol	356	229		603	594							
vCu, unblocked vol	950	824	347	818	809	215	347			229		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	99	99	93	100	83	100			91		
cM capacity (veh/h)	343	411	696	410	422	825	1212			1339		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	14	29	141	229	124	347						
Volume Left	2	29	0	0	124	0						
Volume Right	6	0	141	29	0	0						
cSH	476	410	825	1212	1339	1700						
Volume to Capacity	0.03	0.07	0.17	0.00	0.09	0.20						
Queue Length 95th (ft)	2	6	15	0	8	0						
Control Delay (s)	12.8	14.5	10.3	0.0	8.0	0.0						
Lane LOS	В	В	В		А							
Approach Delay (s)	12.8	11.0		0.0	2.1							
Approach LOS	В	В										
Intersection Summary												
Average Delay			3.4									
Intersection Capacity Utilizat	tion		43.4%	IC	U Level	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		- ሽ	4î		ሻ	4	
Volume (veh/h)	25	1	10	15	0	50	5	285	5	30	380	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	29	1	12	18	0	59	6	335	6	35	447	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (ft)											1102	
pX, platoon unblocked												
vC, conflicting volume	924	871	448	880	869	338	448			341		
vC1, stage 1 conf vol	518	518		350	350							
vC2, stage 2 conf vol	406	353		530	519							
vCu, unblocked vol	924	871	448	880	869	338	448			341		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	93	100	98	96	100	92	99			97		
cM capacity (veh/h)	416	448	611	444	452	704	1112			1218		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	42	76	6	341	35	448						
Volume Left	29	18	6	0	35	0						
Volume Right	12	59	0	6	0	1						
cSH	458	620	1112	1700	1218	1700						
Volume to Capacity	0.09	0.12	0.01	0.20	0.03	0.26						
Queue Length 95th (ft)	8	10	0	0	2	0						
Control Delay (s)	13.7	11.6	8.3	0.0	8.0	0.0						
Lane LOS	В	В	А		А							
Approach Delay (s)	13.7	11.6	0.1		0.6							
Approach LOS	В	В										
Intersection Summary												
Average Delay			1.9									
Intersection Capacity Utiliza	ation		36.7%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>††</b>	1	٦	<u></u>	1	۲.	•	1	٦	•	1
Volume (veh/h)	65	1275	325	115	1305	305	265	175	105	265	205	95
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	195	1898	837	216	1959	864	347	277	234	368	277	234
Arrive On Green	0.04	0.51	0.51	0.06	0.53	0.53	0.14	0.15	0.15	0.14	0.15	0.15
Sat Flow, veh/h	1845	3725	1643	1774	3725	1644	1774	1863	1571	1774	1863	1571
Grp Volume(v), veh/h	73	1433	365	129	1466	343	298	197	118	298	230	107
Grp Sat Flow(s),veh/h/ln	1845	1863	1643	1774	1863	1644	1774	1863	1571	1774	1863	1571
Q Serve(g_s), s	2.2	36.1	16.5	3.8	36.2	14.7	16.8	11.9	8.1	16.8	14.1	7.3
Cycle Q Clear(g_c), s	2.2	36.1	16.5	3.8	36.2	14.7	16.8	11.9	8.1	16.8	14.1	7.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	195	1898	837	216	1959	864	347	277	234	368	277	234
V/C Ratio(X)	0.37	0.75	0.44	0.60	0.75	0.40	0.86	0.71	0.51	0.81	0.83	0.46
Avail Cap(c_a), veh/h	245	1898	837	235	1959	864	347	293	247	368	293	247
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.3	23.0	18.2	22.5	21.8	16.7	36.5	47.7	46.1	36.2	48.7	45.8
Incr Delay (d2), s/veh	1.2	2.8	1.6	3.6	2.7	1.4	18.9	7.4	1.7	12.7	17.2	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/In	1.0	16.5	6.6	4.0	16.4	5.9	10.8	6.3	3.3	8.8	8.2	3.0
Lane Grp Delay (d), s/veh	20.4	25.9	19.9	26.1	24.5	18.1	55.4	55.1	47.8	48.9	65.9	47.2
Lane Grp LOS	С	C	В	С	С	В	E	E (10	D	D	E	D
Approach Vol, veh/h		1871			1938			613			635	
Approach Delay, s/veh		24.5			23.5			53.8			54.8	
Approach LOS		С			С			D			D	
Timer												
Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.8	65.0		9.7	66.9		20.0	23.0		20.0	23.0	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	7.0	59.0		7.0	59.0		16.0	17.5		16.0	17.5	
Max Q Clear Time (g_c+I1), s	4.2	38.1		5.8	38.2		18.8	13.9		18.8	16.1	
Green Ext Time (p_c), s	0.0	16.5		0.0	16.4		0.0	0.9		0.0	0.4	
Intersection Summary												
HCM 2010 Ctrl Delay			31.4									
HCM 2010 LOS			С									
Notes												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	4			4		ሻ	4	
Volume (veh/h)	20	5	10	20	10	80	15	170	25	85	95	30
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	24	6	12	24	12	94	18	200	29	100	112	35
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	679	594	129	576	597	215	147			229		
vC1, stage 1 conf vol	329	329		250	250							
vC2, stage 2 conf vol	350	265		326	347							
vCu, unblocked vol	679	594	129	576	597	215	147			229		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	99	99	96	98	89	99			93		
cM capacity (veh/h)	436	508	920	551	522	825	1435			1339		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	41	24	106	247	100	147						
Volume Left	24	24	0	18	100	0						
Volume Right	12	0	94	29	0	35						
cSH	525	551	775	1435	1339	1700						
Volume to Capacity	0.08	0.04	0.14	0.01	0.07	0.09						
Queue Length 95th (ft)	6	3	12	1	6	0						
Control Delay (s)	12.4	11.8	10.4	0.6	7.9	0.0						
Lane LOS	В	В	В	A	A							
Approach Delay (s)	12.4	10.6	-	0.6	3.2							
Approach LOS	В	В										
Intersection Summary												
Average Delay			4.3									
Intersection Capacity Utilizat	tion		36.8%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									
<i>j </i>												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			- ↔		<u>۲</u>	eî 👘		<u>۲</u>	ef 👘	
Volume (veh/h)	30	0	0	1	0	10	5	265	5	10	205	40
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	35	0	0	1	0	12	6	312	6	12	241	47
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (ft)											1102	
pX, platoon unblocked												
vC, conflicting volume	624	618	265	591	638	315	288			318		
vC1, stage 1 conf vol	288	288		326	326							
vC2, stage 2 conf vol	335	329		265	312							
vCu, unblocked vol	624	618	265	591	638	315	288			318		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	94	100	100	100	100	98	100			99		
cM capacity (veh/h)	568	551	774	592	547	726	1274			1242		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	35	13	6	318	12	288						
Volume Left	35	13	6	0	12	200						
	0	12	0	6	0	47						
Volume Right cSH	568	711	1274	1700	1242	47						
	0.06	0.02	0.00	0.19	0.01	0.17						
Volume to Capacity	0.06					0.17						
Queue Length 95th (ft)	11.8	1 10.2	0 7.8	0 0.0	1 7.9	0.0						
Control Delay (s) Lane LOS				0.0		0.0						
Approach Delay (s)	B 11.8	B 10.2	A 0.1		A 0.3							
Approach LOS	11.8 B	10.2 B	0.1		0.3							
	В	ט										
Intersection Summary			1.0									
Average Delay	tion		1.0			of Comiles			٨			
Intersection Capacity Utiliza	lion		29.2%	IC	U Level (	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>^</b>	1	٦	<u>^</u>	1	٦	•	1	٦	•	7
Volume (veh/h)	55	925	155	65	755	255	135	115	55	130	75	70
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	0.99		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	412	2240	989	338	2246	991	305	182	152	280	182	152
Arrive On Green	0.04	0.60	0.60	0.04	0.60	0.60	0.10	0.10	0.10	0.10	0.10	0.10
Sat Flow, veh/h	1845	3725	1644	1774	3725	1644	1774	1863	1564	1774	1863	1564
Grp Volume(v), veh/h	65	1088	182	76	795	300	142	125	65	153	88	82
Grp Sat Flow(s),veh/h/ln	1845	1863	1644	1774	1863	1644	1774	1863	1564	1774	1863	1564
Q Serve(g_s), s	1.3	17.0	5.1	1.6	11.1	9.1	7.2	6.7	4.0	7.8	4.6	5.1
Cycle Q Clear(g_c), s	1.3	17.0	5.1	1.6	11.1	9.1	7.2	6.7	4.0	7.8	4.6	5.1
Prop In Lane	1.00	0040	1.00	1.00	00.47	1.00	1.00	100	1.00	1.00	100	1.00
Lane Grp Cap(c), veh/h	412	2240	989	338	2246	991	305	182	152	280	182	152
V/C Ratio(X)	0.16	0.49	0.18	0.22	0.35	0.30	0.47	0.69	0.43	0.55	0.48	0.54
Avail Cap(c_a), veh/h	495	2240	989	414	2246	991	305	226	190	280	226	190
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	7.4 0.2	11.6	9.2 0.4	8.4	10.3 0.4	9.9	36.5	45.0	43.8	36.9	44.1	44.3
Incr Delay (d2), s/veh Initial Q Delay(d3),s/veh	0.2	0.8 0.0	0.4	0.3 0.0	0.4	0.8 0.0	1.1 0.0	6.3 0.0	1.9 0.0	2.2 0.0	2.0 0.0	2.9 0.0
%ile Back of Q (50%), veh/ln	0.0	7.2	0.0 1.9	0.0	4.6	0.0 3.5	3.3	3.5	1.6	3.7	2.3	2.1
Lane Grp Delay (d), s/veh	7.6	12.3	9.6	8.8	10.8	10.7	37.6	51.3	45.7	39.0	46.1	47.2
Lane Grp LOS	7.0 A	12.3 B	9.0 A	0.0 A	10.8 B	10.7 B	37.0 D	51.5 D	43.7 D	39.0 D	40.1 D	47.2 D
Approach Vol, veh/h		1335			1171	D	U	332	U	U	323	
Approach Delay, s/veh		1335			10.6			44.3			43.0	
Approach LOS		B			10.0 B			44.3 D			43.0 D	
		D			D			U			U	
Timer Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.4	4 67.0		3 7.5	67.2		13.0	15.6		13.0	15.6	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	4.0	61.0		4.0 8.0	61.0		4.0 9.0	0.5 11.5		4.0 9.0	0.5 11.5	
Max Q Clear Time $(g_c+11)$ , s		19.0		3.6	13.1		9.0	8.7		9.0	7.1	
Green Ext Time (p_c), s	0.0	13.7		0.1	14.1		9.2 0.0	0.4		0.0	0.5	
Intersection Summary												
HCM 2010 Ctrl Delay			17.9									
HCM 2010 LOS			В									
Notes												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		<u>۲</u>	4			4		ሻ	4	
Volume (veh/h)	30	10	15	25	5	120	10	170	25	105	295	20
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	35	12	18	29	6	141	12	200	29	124	347	24
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			TWLTL	
Median storage veh)											2	
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	988	859	359	856	856	215	371			229		
vC1, stage 1 conf vol	606	606		238	238							
vC2, stage 2 conf vol	382	253		618	618							
vCu, unblocked vol	988	859	359	856	856	215	371			229		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	89	97	97	92	99	83	99			91		
cM capacity (veh/h)	326	401	686	380	401	825	1188			1339		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	SB 1	SB 2						
Volume Total	65	29	147	241	124	371						
Volume Left	35	29	0	12	124	0						
Volume Right	18	0	141	29	0	24						
cSH	396	380	792	1188	1339	1700						
Volume to Capacity	0.16	0.08	0.19	0.01	0.09	0.22						
Queue Length 95th (ft)	14	6	17	1	8	0						
Control Delay (s)	15.8	15.3	10.6	0.5	8.0	0.0						
Lane LOS	С	С	В	А	A							
Approach Delay (s)	15.8	11.4		0.5	2.0							
Approach LOS	С	В										
Intersection Summary												
Average Delay			4.2									
Intersection Capacity Utiliza	tion		52.1%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		1	ef 👘		ሻ	eî 👘	
Volume (veh/h)	50	1	10	15	0	50	5	310	5	30	400	25
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	59	1	12	18	0	59	6	365	6	35	471	29
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (ft)											1102	
pX, platoon unblocked												
vC, conflicting volume	991	938	485	933	950	368	500			371		
vC1, stage 1 conf vol	556	556		379	379							
vC2, stage 2 conf vol	435	382		554	571							
vCu, unblocked vol	991	938	485	933	950	368	500			371		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	85	100	98	96	100	91	99			97		
cM capacity (veh/h)	393	427	582	426	426	678	1064			1188		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	72	76	6	371	35	500						
Volume Left	59	18	6	0	35	0						
Volume Right	12	59	0	6	0	29						
cSH	416	597	1064	1700	1188	1700						
Volume to Capacity	0.17	0.13	0.01	0.22	0.03	0.29						
Queue Length 95th (ft)	15	11	0	0	2	0						
Control Delay (s)	15.4	11.9	8.4	0.0	8.1	0.0						
Lane LOS	С	В	А		А							
Approach Delay (s)	15.4	11.9	0.1		0.5							
Approach LOS	С	В										
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utilizati	ion		41.4%	IC	U Level o	of Service			А			
Analysis Period (min)			15									
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	- <b>††</b>	1	<u> </u>	- <b>††</b>	1	<u>۲</u>	<b>↑</b>	1	<u> </u>	<b>↑</b>	1
Volume (veh/h)	65	1275	355	130	1305	305	295	180	120	265	210	95
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	193.7	186.3	193.7	186.3	186.3	193.7	186.3	186.3	186.3	186.3	186.3	186.3
Lanes	1	2	1	1	2	1	1	1	1	1	1	1
Cap, veh/h	195	1884	831	219	1959	864	343	281	237	364	281	237
Arrive On Green	0.04	0.51	0.51	0.06	0.53	0.53	0.14	0.15	0.15	0.14	0.15	0.15
Sat Flow, veh/h	1845	3725	1643	1774	3725	1644	1774	1863	1571	1774	1863	1571
Grp Volume(v), veh/h	73	1433	399	146	1466	343	331	202	135	298	236	107
Grp Sat Flow(s),veh/h/ln	1845	1863	1643	1774	1863	1644	1774	1863	1571	1774	1863	1571
Q Serve(g_s), s	2.2	36.7	18.8	4.3	36.5	14.8	17.0	12.3	9.5	16.9	14.6	7.4
Cycle Q Clear(g_c), s	2.2	36.7	18.8	4.3	36.5	14.8	17.0	12.3	9.5	16.9	14.6	7.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	195	1884	831	219	1959	864	343	281	237	364	281	237
V/C Ratio(X)	0.37	0.76	0.48	0.67	0.75	0.40	0.96	0.72	0.57	0.82	0.84	0.45
Avail Cap(c_a), veh/h	244	1884	831	230	1959	864	343	290	245	364	290	245
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.4	23.5	19.1	23.6	22.0	16.9	38.9	48.0	46.8	36.4	49.0	45.9
Incr Delay (d2), s/veh	1.2	3.0	2.0	6.7	2.7	1.4	38.9	8.1	2.9	13.7	18.8	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/In	1.0	16.9	7.6	4.8	16.8	6.0	3.7	6.5	3.9	9.0	8.5	3.0
Lane Grp Delay (d), s/veh	20.6	26.5	21.1	30.3	24.7	18.2	77.8	56.0	49.7	50.1	67.8	47.2
Lane Grp LOS	С	С	С	С	С	В	E	E	D	D	E	<u>D</u>
Approach Vol, veh/h		1905			1955			668			641	
Approach Delay, s/veh		25.1			24.0			65.6			56.1	
Approach LOS		С			С			E			E	
Timer												
Assigned Phs	7	4		3	8		1	6		5	2	
Phs Duration (G+Y+Rc), s	7.9	65.0		10.2	67.4		20.0	23.4		20.0	23.4	
Change Period (Y+Rc), s	4.0	6.0		4.0	6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s	7.0	59.0		7.0	59.0		16.0	17.5		16.0	17.5	
Max Q Clear Time (g_c+I1), s	4.2	38.7		6.3	38.5		19.0	14.3		18.9	16.6	
Green Ext Time (p_c), s	0.0	16.3		0.0	16.4		0.0	0.9		0.0	0.3	
Intersection Summary												
HCM 2010 Ctrl Delay			33.8									
HCM 2010 LOS			С									
Notes												

\_\_\_\_\_

Count Data Sheets

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North/South Street: Boardwalk East/West Street: Harmony Time: AM ICU Number: 99 File Name : Boardwalk & Harmony 7-9-13 Site Code : 00000099 Start Date : 7/9/2013 Page No : 1

						(	Group	s Printed	l- Unsh	ifted							
			dwalk bound			Harn Westb	nony bound				dwalk bound	1			nony bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
07:30 AM	16	12	29	57	58	164	9	231	9	17	15	41	21	208	12	241	570
07:45 AM	12	22	34	68	79	210	9	298	12	36	25	73	31	267	19	317	756
Total	28	34	63	125	137	374	18	529	21	53	40	114	52	475	31	558	1326
08:00 AM	20	21	36	77	65	197	14	276	5	20	29	54	33	216	8	257	664
08:15 AM	17	14	26	57	46	165	17	228	13	36	37	86	38	210	12	260	631
Grand Total	65	69	125	259	248	736	49	1033	39	109	106	254	123	901	51	1075	2621
Apprch %	25.1	26.6	48.3		24	71.2	4.7		15.4	42.9	41.7		11.4	83.8	4.7		
Total %	2.5	2.6	4.8	9.9	9.5	28.1	1.9	39.4	1.5	4.2	4	9.7	4.7	34.4	1.9	41	



North/South Street: Boardwalk East/West Street: Harmony Time: NN ICU Number: 99 File Name : Boardwalk & Harmony 7-9-13 Site Code : 00000099 Start Date : 7/9/2013 Page No : 1

						(	Group	s Printed	l- Unsh	ifted							
		Boar South	dwalk bound	ł			nony bound			Board	dwalk bound	l.		Harn Eastb	nony oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
12:00 PM	22	48	67	137	78	275	26	379	24	66	66	156	74	259	16	349	1021
12:15 PM	21	55	62	138	78	292	17	387	17	35	59	111	68	328	13	409	1045
12:30 PM	28	47	62	137	61	222	19	302	20	60	61	141	74	288	19	381	961
12:45 PM	21	56	75	152	88	282	20	390	23	56	68	147	90	331	19	440	1129
Total	92	206	266	564	305	1071	82	1458	84	217	254	555	306	1206	67	1579	4156
Grand Total Apprch %	92 16.3	206 36.5	266 47.2	564	305 20.9	1071 73.5	82 5.6	1458	84 15.1	217 39.1	254 45.8	555	306 19.4	1206 76.4	67 4.2	1579	4156
Total %	2.2	5	6.4	13.6	7.3	25.8	2	35.1	2	5.2	6.1	13.4	7.4	29	1.6	38	



North/South Street: Boardwalk East/West Street: Harmony Time: PM ICU Number: 99 File Name : Boardwalk & Harmony 7-9-13 Site Code : 00000099 Start Date : 7/9/2013 Page No : 1

							Group	s Printed	l- Unsh	ifted							
			dwalk				nony bound				dwalk bounc				nony bound		
Start Time	Right	Thru	Left	1	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:30 PM	29	41	61	131	70	267	24	361	21	39	73	133	70	296	17	383	1008
04:45 PM	17	46	58	121	72	333	21	426	25	30	56	111	73	354	17	444	1102
Total	46	87	119	252	142	600	45	787	46	69	129	244	143	650	34	827	2110
05:00 PM	26	61	75	162	76	356	25	457	32	60	63	155	74	280	12	366	1140
05:15 PM	22	53	64	139	79	318	40	437	26	44	66	136	100	314	14	428	1140
Grand Total	94	201	258	553	297	1274	110	1681	104	173	258	535	317	1244	60	1621	4390
Apprch %	17	36.3	46.7		17.7	75.8	6.5		19.4	32.3	48.2		19.6	76.7	3.7		
Total %	2.1	4.6	5.9	12.6	6.8	29	2.5	38.3	2.4	3.9	5.9	12.2	7.2	28.3	1.4	36.9	



North/South Street: Boardwalk East/West Street: Harmony Time: PHF ICU Number: 99 
 File Name
 : Boardwalk & Harmony 7-9-13

 Site Code
 : 00000099

 Start Date
 : 7/9/2013

 Page No
 : 1

		South	dwalk bounc				nony bound				dwalk bound			East	nony bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Tota
Peak Hour Ana	alysis Fr	om 07:3	30 AM t	o 09:45 A	M - Pea	ak 1 of 1											
Peak Hour for																	
07:30 AM	16	12	29	57	58	164	9	231	9	17	15	41	21	208	12	241	570
07:45 AM	12	22	34	68	79	210	9	298	12	36	25	73	31	267	19	317	756
08:00 AM	20	21	36	77	65	197	14	276	5	20	29	54	33	216	8	257	664
08:15 AM	17	14	26	57	46	165	17	228	13	36	37	86	38	210	12	260	631
<b>Total Volume</b>	65	69	125	259	248	736	49	1033	39	109	106	254	123	901	51	1075	2621
% App. Total	25.1	26.6	48.3		24	71.2	4.7		15.4	42.9	41.7		11.4	83.8	4.7		
PHF	.813	.784	.868	.841	.785	.876	.721	.867	.750	.757	.716	.738	.809	.844	.671	.848	.867
12:00 PM 12:15 PM 12:30 PM 12:45 PM Total Volume	22 21 <b>28</b> 21 92	48 55 47 <b>56</b> 206	67 62 62 <b>75</b> 266	137 138 137 <b>152</b> 564	78 78 61 <b>88</b> 305	275 <b>292</b> 222 282 1071	26 17 19 20 82	379 387 302 <b>390</b> 1458	24 17 20 23 84	66 35 60 56 217	66 59 61 <b>68</b> 254	<b>156</b> 111 141 147 555	74 68 74 <b>90</b> 306	259 328 288 <b>331</b> 1206	16 13 <b>19</b> 19 67	349 409 381 <b>440</b> 1579	1021 1045 961 <b>1129</b> 4156
% App. Total	16.3	36.5	47.2	504	20.9	73.5	5.6	1450	15.1	39.1	45.8	555	19.4	76.4	4.2	1579	4150
PHF	.821	.920	.887	.928	.866	.917	.788	.935	.875	.822	.934	.889	.850	.911	.882	.897	.920
Peak Hour Ana Peak Hour for I 04:30 PM 04:45 PM				ins at 04: 131 121 <b>162</b>	30 PM 70 72 76	267 333 <b>356</b>	24 21 25 <b>40</b>	361 426 <b>457</b> 437	21 25 <b>32</b> 26	39 30 <b>60</b> 44	<b>73</b> 56 63 66	133 111 <b>155</b> 136	70 73 74 <b>100</b>	296 <b>354</b> 280 314	<b>17</b> 17 12 14	383 444 366 428	1008 1102 <b>1140</b> 1140
05:00 PM 05:15 PM Total Volume	20 22 94	53 201	64 258	139 553	<b>79</b> 297	318 1274	110	1681	104	173	258	535	317	1244	60	1621	
05:00 PM 05:15 PM	22			and the second second second second													4390









Combined total for AM - Noon and PM

#### COUNTER MEASURES INC. 1889 YORK STREET DENVER COLORADO 80206

N/S STREET: BOARDWALK DR E/W STREET: SAMS CLUB / MIRAMONT O-P ACC CITY: FORT COLLINS COUNTY: LARIMER

DENVER,COLORADO 80206 303-333-7409

File Name : BROASAMS Site Code : 0000005 Start Date : 9/17/2013 Page No : 1

NTT: LARIVIER					Groups	Printed- VI	EHICLES				Pag	eNo :	1
		DWALK uthbound			LUB ACC	·····	BOAR	DWALK	DR	MIRAM PAR Ea			
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Tota
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
07:00 AM	0	22	1	0	0	0	1	65	0	0	0	0	89
07:15 AM	0	25	1	0	0	0	0	55	0	0	0	0	8
07:30 AM	0	27	2	0	0	1	1	61	· 0	0	0	0	92
07:45 AM	1	53	6	0	0	0	1	66	0	1	0	Ō	128
Total	1	127	10	0	0	1	3	247	0	1	0	0	390
08:00 AM	2	37	4	0	0	2	0	61	0	2	0	0	108
08:15 AM	1	43	1	0	0	2	0	57	0	0	0	0	104
08:30 AM	2	43	2	1	0	6	2	56	3	1	0	0	116
08:45 AM	6	43	1	3	0	0	1	62	1	4	0	1	122
Total	11	166	8	4	0	10	3	236	4	7	0	1	45(
04:00 PM	10	62	3	5	0	15	0	51	4	17	0	4	17
04:15 PM	6	64	1	2	0	16	0	61	1	4	0	2	157
04:30 PM	9	58	2	2	0	15	0	62	2	3	0	2	158
04:45 PM	12	81	2	5	0	11	2	63	5	4	0	1	186
Total	37	265	8	14	0	57	2	237	12	28	0	9	669
05:00 PM	4	86	0	2	0	10	1	61	1	13	1	4	18:
05:15 PM	11	102	0	1	0	11	0	66	1	2	0	3	197
05:30 PM	5	102	0	2	0	11	1	79	3	7	0	0	21(
05:45 PM	9	81	1	6	0	15	3	70	0	1	0	2	188
Total	29	371	1	11	0	47	5	276	5	23	1	9	778
Grand Total	78	929	27	29	0	115	13	996	21	59	1	19	2287
Apprch %	7.5	89.8	2.6	20.1	0.0	79.9	1.3	96.7	2.0	74.7	1.3	24.1	
Total %	3.4	40.6	1.2	1.3	0.0	5.0	0.6	43.6	0.9	2.6	0.0	0.8	

N/S STREET: BOARDWALK DR E/W STREET: SAMS CLUB / MIRAMONT O-P ACC CITY: FORT COLLINS COUNTY: LARIMER

#### COUNTER MEASURES INC. 1889 YORK STREET DENVER,COLORADO 80206 303-333-7409

File Name : BROASAMS Site Code : 0000005 Start Date : 9/17/2013 Page No : 2

	E		WALK I	DR	SAM CLUB ACCESS Westbound				B		WALK [ nbound	DR	MIRA				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	om 07:0	0 AM to	09:00	AM - Pea	ik 1 of 1				·								
Intersection	07:45	AM															
Volume	6	176	13	195	1	0	10	11	3	240	3	246	4	0	0	4	456
Percent	3.1	90.3	6.7		9.1	0.0	90.9		1.2	97.6	1.2		100. 0	0.0	0.0		
07:45 Volume	1	53	6	60	0	0	0	0	1	66	0	67	1	0	0	1	128
Peak Factor																	0.891
High Int.					08:30 AM				07:45	AM			08:00				
Volume	1	53	6	60	1	0	6	7	1	66	0	67	2	0	0	2	
Peak Factor				0.813				0.393				0.918				0.500	



N/S STREET: BOARDWALK DR E/W STREET: SAMS CLUB / MIRAMONT O-P ACC CITY: FORT COLLINS COUNTY: LARIMER

#### COUNTER MEASURES INC. 1889 YORK STREET DENVER,COLORADO 80206 303-333-7409

File Name : BROASAMS Site Code : 0000005 Start Date : 9/17/2013 Page No : 2

	B		WALK I	DR	SAM CLUB ACCESS Westbound				B		WALK [ nbound	)R	MIRA				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	m 04:0	0 PM to	05:45	PM - Pea	ik 1 of 1						·						
Intersection	05:00	PM															
Volume	29	371	1	401	11	0	47	58	5	276	5	286	23	1	9	33	778
Percent	7.2	92.5	0.2		19.0	0.0	81.0		1.7	96.5	1.7		69.7	3.0	27.3		
05:30 Volume	5	102	0	107	2	0	11	13	1	79	3	83	7	0	0	7	210
Peak Factor																	0.926
High Int.				05:45 PM			05:30 PM				05:00						
Volume Peak Factor	11	102	0	113 0.887	6	0	15	21 0.690	1	79	3	83 0.861	13	1	4	18 0.458	



#### COUNTER MEASURES INC. 1889 YORK STREET DENVER,COLORADO 80206 303-333-7409

N/S STREET: BROADWAY DR E/W STREET: OAKRIDGE DR CITY: FORT COLLINS COUNTY: LARIMER

#### File Name : BROAOAKR Site Code : 00000010 Start Date : 9/17/2013 Page No : 1

NIT: LARIMER					Groupe	Printed- VI					Pag	eNo :	1
	BROA		DR	OAK	RIDGE D	R		DWALK	DR	OAK		P	
	So	uthbound			estbound			orthbound			astbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Tota
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
07:00 AM	13	8	1	2	0	9	0	57	3	0	0	0	93
07:15 AM	14	11	0	1	0	12	0	43	3	0	0	Ō	84
07:30 AM	15	12	0	3	0	19	0	43	5	0	0	Ō	97
07:45 AM	23	28	2	3	2	20	3	47	6	0	0	ō	134
Total	65	59	3	9	2	60	3	190	17	0	0	0	408
08:00 AM	16	21	0	2	1	21	1	40	2	0	0	0	104
08:15 AM	23	20	0	7	0	17	0	40	7	0	0	0	114
08:30 AM	20	24	0	6	0	21	0	40	8	0	1	1	121
08:45 AM	24	23	0	1	0	15	2	49	9	0	Ó	Ó	123
Total	83	88	0	16	1	74	3	169	26	0	1	1	462
04:00 PM	22	49	0	0	0	10						- 1	
04:15 PM	20	49 48	0	9 11	0	16	2	38	8	1	0	2	147
04:30 PM	20 19	40 43	0	13	0	25	0	37	4	Ó	Q	2	147
04:45 PM	20	43 67	0	13	0	25	0	38	3	1	1	0	143
Total	81	207	0	43	0	27 93	0	43	5	0	0		173
	01	201	0	43	0	93	2	156	20	2	1	5	610
05:00 PM	19	73	0	7	0	29	0	32	8	2	3	3	176
05:15 PM	25	81	0	5	0	24	0	43	6	0	Ō	ō	184
05:30 PM	32	72	0	6	0	34	0	49	8	Ō	ō	ō	201
05:45 PM	27	62	0	7	0	31	0	42	4	0	Ō	ō	173
Total	103	288	0	25	0	118	0	166	26	2	3	3	734
Grand Total	332	642	3	93	3	345	8	681	89	4	5	9	2214
Apprch %	34.0	65.7	0.3	21.1	0.7	78.2	1.0	87.5	11.4	22.2	27.8	50.0	
Total %	15.0	29.0	0.1	4.2	0.1	15.6	0.4	30.8	4.0	0.2	0.2	0.4	

#### COUNTER MEASURES INC. 1889 YORK STREET DENVER,COLORADO 80206 303-333-7409

N/S STREET: BROADWAY DR E/W STREET: OAKRIDGE DR CITY: FORT COLLINS COUNTY: LARIMER

#### File Name : BROAOAKR Site Code : 0000010 Start Date : 9/17/2013 Page No : 2

	В		WALK [	DR			DGE DI tbound	٦	B		WALK E	DR					
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	m 07:00	O AM to	09:00	AM - Pea	ak 1 of 1		·		···				I				
Intersection	07:45	AM			ļ				1							1	
Volume	82	93	2	177	18	3	79	100	4	167	23	194	0	1	1	2	473
Percent	46.3	52.5	1.1		18.0	3.0	79.0		2.1	86.1	11.9		0.0	50.0	50.0	-	470
07:45 Volume	23	28	2	53	3	2	20	25	3	47	6	56	0	0	0	0	134
Peak Factor					i de la constante de la consta												0.882
0	07:45 AM			08:30 AM			07:45 AM				08:30	0.002					
Volume	23	28	2	53	6	0	21	27	3	47	6	56	0	1	1	2	
Peak Factor				0.835				0.926				0.866				0.250	



N/S STREET: BROADWAY DR E/W STREET: OAKRIDGE DR CITY: FORT COLLINS COUNTY: LARIMER

#### COUNTER MEASURES INC. 1889 YORK STREET DENVER,COLORADO 80206 303-333-7409

File Name : BROAOAKR Site Code : 0000010 Start Date : 9/17/2013 Page No : 2

	B		WALK [		OAKRIDGE DR Westbound						WALK D	R					
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	om 05:00	0 PM to	05:45	PM - Pea	ak 1 of 1								·				
Intersection	05:00	РM														1	
Volume	103	288	0	391	25	0	118	143	0	166	26	192	2	3	3	8	734
Percent	26.3	73.7	0.0		17.5	0.0	82.5	_	0.0	86.5	13.5		25.0	37.5	37.5	Ŭ	
05:30 Volume	32	72	0	104	6	0	34	40	0	49	8	57	0	0	0	0	201
Peak Factor																	0.040
High Int.	05:15 PM				05:30 PM				05:30 PM				05:00 PM				0.913
Volume	25	81	0	106	6	0	34	40	0	49	8	57	2	3	3	8	
Peak Factor				0.922				0.894				0.842				0.250	

