

ITEM NO FDP #130021

MEETING DATE July 23, 2013

STAFF Pete Wray

ADMINISTRATIVE TYPE I HEARING

STAFF REPORT

PROJECT: Harmony Technology Park Third Filing, Second Replat – Custom

Blending, Project Development Plan/Final Development Plan - FDP

#130021

APPLICANT: Cathy Mathis

TB Group

444 Mountain Ave. Berthoud, CO

OWNER: Custom Blending

3461 Precision Drive Fort Collins, CO 80528

PROJECT DESCRIPTION:

This is a request is for a combined Project Development Plan/Final Plan for a building expansion to the Custom Blending facility located at 3461 Precision Drive. Custom Blending is a manufacturer of spices, seasonings and flavoring extracts and the impetus for the expansion is to create space primarily for the manufacture of extracts in-house. The approximate 31,700 square foot existing Custom Blending building is located on 3 acres, in Lot 2 in the Harmony Technology Park Third Filing.

The proposed addition to the east of the existing building would contain approximately 35,000 square feet, and expand into a portion of Lot 3. This project will include replat to combine Lot 2 and Lot 3. The proposed expansion is anticipated to be 40-50% production/warehouse and approximately 3,000 to 5,000 square feet of office space. The new building will be expanding over the existing 44-space employee parking lot. Project build-out will provide new access drives on the south and east sides of the building and new parking area on the east side of the building.

RECOMMENDATION: Approval of the FDP

EXECUTIVE SUMMARY:

The FDP complies with procedural requirements of the Land Use Code (LUC) in Article 2; the expansion and replat are subject to Administrative (Type 1) review. The FDP

complies with the applicable zoning standards of LUC Section 4.26 – Harmony Corridor District. And, the FDP complies with the General Development standards in Article 3.

COMMENTS:

1. **Zoning History**:

- The property was annexed in 1994 as part of the 155 acre Harmony Farm Annexation and zoned Harmony Corridor (HC) District.
- The property including Lot 2 and 3 is part of the Harmony Technology Park, Third Filing, and Second Replat, last amended in 2008.

Current surrounding zoning and land uses are as follows:

Direction	Zone District	Existing Land Uses
North	Harmony Corridor (HC)	Vacant land, Lot 2, Harmony Tech. Park, Second Filing
South	Harmony Corridor (HC) Low Density Mixed-use Neighborhood (LMN)	Vacant land, Tract A, Harmony Tech. Park, Third Filing – Drainage/Utility Easement Fossil Ridge H. S., Rock Creek Dr. South Side
East	Harmony Corridor (HC)	Vacant land, Lot 4, Harmony Tech. Park, Third Filing
West	Harmony Corridor (HC)	Vacant land, Lot 1, Harmony Tech. Park, Third Filing, First Replat

2. <u>Compliance with Article 3 of the Land Use Code - General Development Standards</u>

Staff finds that the project complies with all applicable Article 3 standards. Following are some key examples:

Division 3.2 – Site Planning and Design Standards

A. Section 3.2.1 (D) – Tree Planting Standards

These standards require full tree stocking and street trees included in landscape plan.

The proposed landscape plan complies with the standard in that landscape areas are provided in adequate numbers, locations, and dimensions to allow full tree stocking along all high use or high visibility

sides of buildings per Section 3.2.1 (D)(1)(c). The proposed landscape plan provides deciduous shade trees in two varieties within the parkway along Precision Drive and spaced at intervals ranging between 30 and 35 feet in compliance with the standard in Section 3.2.1(D)(2)(a).

B. Section 3.2.1(E) – Interior and Parking Lot Perimeter Landscaping

This standard requires perimeter parking areas are screened from the street and abutting uses by providing sufficient ground plane and tree canopy plantings to screen at least 75% of the light from headlights, to a minimum height of 30" for at least 70% of the length of the parking area along the street.

The landscape plan complies with the landscape standards for perimeter, building foundation, and interior parking-island planting.

C. Section 3.2.2 (C) – Development Standards

This standard requires to the maximum extent feasible separation of pedestrians and vehicles, direct pedestrian connections to street, and provide bicycle facilities.

The proposed expansion project maintains building orientation fronting Precision Drive with entrances that face and open directly to walkways that connect to public sidewalks along the street. Pedestrian and bicycle movement and access on site are separate from vehicle and truck drive aisles and parking, loading areas. The proposed project provides 4 new bicycle parking spaces (2 existing).

D. Section 3.2.2 (K) (2) Nonresidential Parking Requirements

This standard requires that a maximum number of standard parking spaces, based on the square footage of the gross leasable area and of the occupancy of specified uses, not exceed 45 standard spaces. In addition, two accessible parking spaces and 4 fixed bicycle spaces are required.

The project includes 45 off-street parking spaces and 3 accessible spaces; six bike parking spaces are also provided (2 existing and 4 new) in compliance with Section 3.2.2 (C) (4).

E. Section 3.2.4 – Site Lighting

The Lighting Plan indicates that both pole and building mounted light fixtures will feature fully shielded and down-directional luminaires. In

addition, there will be no illumination exceeding 0.1 (one-tenth) foot-candle as measured 20 feet from the property line.

F. Section 3.2.5 – Trash Enclosures

Trash enclosures are provided in sufficient quantity to serve the project. As required, enclosures will be properly sized to accommodate both trash and recycling containers. Exterior materials will match the buildings.

G. Section 3.4.3 – Water Quality

This standard references the requirements set forth in the *Stormwater Criteria Manual*. The development is required to meet the City's new Low Impact Development (LID) requirements which include 50% of the land area draining through an infiltration device and 25% of all private parking and drive aisles be porous pavement.

Staff determined that the proposed project complies with the LID requirements, and 25% of the parking and drive aisles are not required to be porous pavement, since 50% of the existing site drains through and infiltration device, thus meeting the equal to or better criteria option. All surface stormwater drainage is passed through infiltration devices, prior to entering the existing regional detention pond located in Tract A, part of the Harmony Technology Park overall development plan.

H. Section 3.5.1 – Building and Project Compatibility – Architectural Character

This standard requires that new developments adjacent to existing developed areas shall be compatible with the established architectural character by using a design that is complementary.

This proposed project is surrounded by vacant land within the larger Harmony Technology Park, and compatible complimentary with existing light industrial businesses along Harmony Road to the north.

The proposed building expansion reflects similar architectural exterior façade treatments, mass and scale and building height as the existing building. Exterior materials include precast concrete, 3-coat stucco, and brick veneer to match existing building design and color schemes.

The proposed building expansion and site plan locates truck loading, storage tanks, and trash collection enclosures to the sides and rear of the building and away from the public street, as required in *Section 3.5.1 (I)*.

These facilities are screened from adjacent sites through such means as walls, enclosures, and interior and perimeter landscape plantings.

I. Section 3.5.3 (B) (C) (D) (E) - Mixed-Use, Institutional, and Commercial Buildings

This standard requires architectural design that provides variation in massing and character and interest tailored specifically to the site and its context, orienting building entrances with direct pedestrian connections to the street.

The proposed building expansion maintains overall building orientation to Precision Drive and building entrances face and open directly onto a connecting walkway to the street, without crossing a drive aisle or parking area. The proposed main vehicular access to the expansion site will utilize a new entrance off of Precision Drive. The drive will be 30' wide to accommodate Poudre Fire Authority requirements for a fire lane. Employee parking spaces will be provided to the sides of the existing and proposed building expansion. A truck loading dock and trash enclosure are provided on the south end of the buildings.

The proposed design of the facility expansion will combine the exterior aesthetic of the existing building while incorporating the new process functions of Custom Blending's future. The proposed exterior materials will be identical to the existing facility to create a cohesive addition that presents as a single facility from the exterior. Building wall articulation and façade treatments from the existing building are continued with the proposed expansion, utilizing change in plane, variation of building materials, texture and color. These building treatments provide a distinctive façade base and top that accentuates the entrances facing the street and ground plane. Other side and rear facades differentiate base and top treatments with planters, raised docks, window alignment, walls and variation of building materials and texture, and color.

The proposed building expansion will act a screen from the public R.O.W. by placing required service uses to the south. The addition allows for improved site circulation and access from the existing conditions.

J. Section 3.6.4 – Transportation Level of Service Requirements

This standard requires all development plans shall adequately provide vehicular, pedestrian and bicycle facilities necessary to maintain the adopted transportation level of Service (LOS) standards, and provide a transportation impact study.

A transportation impact study dated June 4, 2013 was provided to City staff. The conclusions form the study show that with full development of the Custom Blending building expansion, the future level of service at key intersections will be acceptable. The proposed building expansion can be built without additional geometry or other street improvements. Based upon Fort Collins bicycle LOS criteria, there are no destination areas within 1,320 feet of the Custom Blending Expansion site. Currently, this area is served by Transfort Routes 16 and 17 and as a result, transit service is acceptable.

3. <u>Compliance with Article 4 of the Land Use Code - General Commercial (CG)</u> <u>District Standards in Division 4.21</u>

Staff finds that the project complies with all applicable Article 4 standards. Following are some key examples:

A. Purpose: The Harmony Corridor District is intended to implement the design concepts and land use vision of the Harmony Corridor Plan -- that of creating an attractive and complete mixed-use area with a major employment base.

The proposed project is consistent with the stated purpose of the zone district, representing a primary use located in an existing employment park, which provides a campus-like setting, with platted lots, streets, sidewalks, and infrastructure already in place.

B. Permitted Land Uses - Section 4.26 (B) (2) (d) 2

The proposed uses of light industrial and offices are permitted in the H-C zoning, subject to Administrative Type I Review, including replatting of lots.

C. Land Use Standards – Section 4.26(D)(1) All development in the H-C Harmony Corridor District shall also comply with the Harmony Corridor design standards as adopted by the city and the following specific standards to the extent that such standards apply to the property proposed for development.

The proposed project complies with the applicable Harmony Corridor Design Standards, included in the LUC.

Land Use/Dimensional Standards – Section 4.26 (D) (3) (a) Maximum height for all nonresidential buildings, including those containing mixed-use dwelling units, shall be six (6) stories.

The proposed building expansion height is 31feet, consistent with the existing building.

Land Use Standards – Section 4.26 (D) (3) (b) All new structures greater than eighty thousand (80,000) square feet in gross leasable area shall be subject to Planning and Zoning Board review.

The proposed building expansion includes a total gross leasable area of approximately 35,000 square feet, subject to an administrative Type I review.

Land Use Standards – Section 4.26 (D) (3) (c) Any building addition that exceeds eighty thousand (80,000) square feet in gross leasable area and exceeds twenty-five (25) percent of the gross leasable area of the existing building shall be subject to Planning and Zoning Board review.

The proposed building expansion includes a total gross leasable area of approximately 35,000 square feet, subject to an administrative Type I review.

D. Development Standards – Section 4.26(E)(2)(c) All commercial/retail and industrial uses, except for off-street parking and loading, shall be conducted or carried out entirely within completely enclosed buildings or structures.

The existing and proposed building expansion does not include any outdoor storage. Two external tank storage areas adjacent to the building are fully enclosed.

E. Building Design – Section 4.26(E)(3)(a) To the extent reasonably feasible, industrial buildings shall provide a primary entrance that faces and opens directly onto the abutting street sidewalk or a walkway, plaza or courtyard that has direct linkage to the street sidewalk without requiring pedestrians to cross any intervening driveways or parking lots.

The existing building and primary entrance that faces and opens north and connecting directly to Precision Drive will be maintained. The proposed building expansion will have a secondary employee entrance with sidewalk connection also to Precision Drive.

4. Neighborhood Meeting:

The LUC does not require that a neighborhood meeting be held for development proposals that are not subject to a Planning and Zoning Board (Type 2) review. Therefore, a City-sponsored and facilitated neighborhood information meeting

was not held for this project. There are no known impacts or issues with any adjacent development.

5. Findings of Fact and Conclusion:

In evaluating Harmony Technology Park Third Filing, Second Replat – Custom Blending, Project Development Plan/Final Development Plan - FDP #130021, staff makes the following findings of fact:

- A. The FDP complies with the process located in Division 2.2 Common Development Review Procedures for Development Applications of Article 2 Administration.
- B. The FDP complies with relevant standards located in Article 3 General Development Standards.
- C. The FDP complies with relevant standards located in Division 4.26, Harmony Corridor of Article 4 Districts.

RECOMMENDATION:

Staff recommends approval of the Harmony Technology Park Third Filing, Second Replat – Custom Blending, Project Development Plan/Final Development Plan - FDP #130021.

ATTACHMENTS:

- 1. Cover Sheet with vicinity map
- 2. Plat Sheet 2
- 3. Site Plan
- 4. Landscape Plan
- 6. Building Elevations
- 7. Traffic Study

UTILITY PLANS FOR HARMONY TECHNOLOGY PARK THIRD FILING, SECOND REPLAT

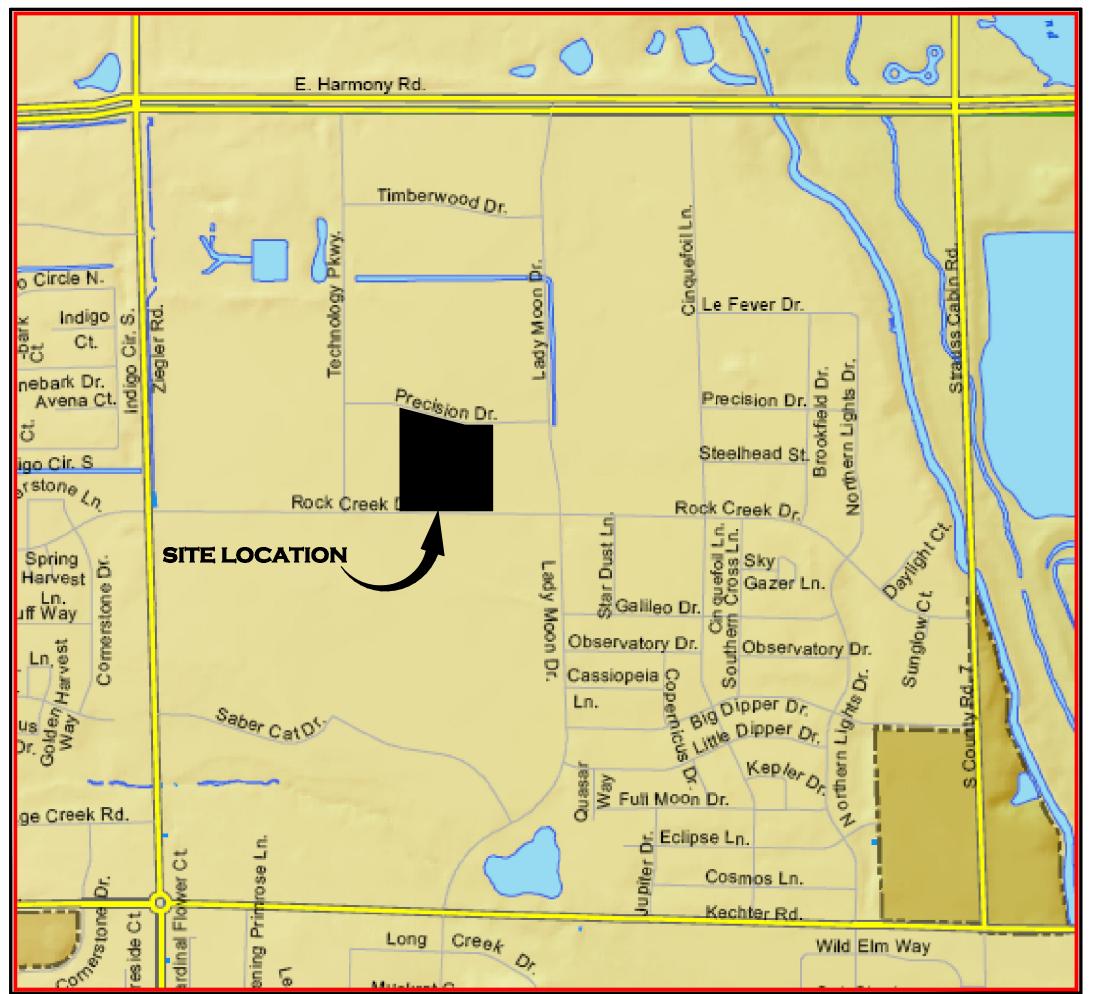
FORT COLLINS, COLORADO

A TRACT OF LAND, BEING A REPLAT OF LOTS 2 & 3 OF THE HARMONY TECHNOLOGY PARK 3RD FILING, LOCATED IN THE NORTHWEST QUARTER OF SECTION 4, TOWNSHIP 6 NORTH, RANGE 68 WEST OF THE 6TH PRINCIPAL MERIDIAN, CITY OF FORT COLLINS, COUNTY OF LARIMER, STATE OF COLORADO

JUNE 19, 2013

PROJECT CONTACTS: Bldg. 6, Suite 100 Fort Collins, CO 80528 Centennial, CO 80111 Berthoud, CO 80513 Contact: Cathy Mathi Fort Collins, CO 80550 Fort Collins, CO 80524 Contact: John Gooch (970) 419-4344

UTILITY ENTITY CONTACTS: CITY OF FORT COLLINS WATER/ XCEL ENERGY - GAS & ELECTRIC Contact: Terry Stencel WASTEWATER Contact: Roger Buffington (970) 221-6854 (970) 225-7848 FORT COLLINS LOVELAND
WATER DISTRICT/ SOUTH FORT CENTURY LINK
Contact: Bob Rulli **COLLINS SANITATION DISTRICT:** (970) 490-7503 Contact: Terry Famili (970) 226-3104 COMCAST Contact: Don Kapperman CITY OF FORT COLLINS STORMWATER (970) 567-0245 Contact: Glen Schleuter (970) 224-6065 POUDRE FIRE AUTHORITY
Contact: Jim Lynxwiler
(970) 416-2869 **CITY OF FORT COLLINS LIGHT** Contact: Rob Irish (970) 224-6167



DRAWING/ SHEET INDEX

7 1 1 2 1 1 1	
SHEET NO.	SHEET TITLE
C-001	COVER SHEET
C-002	GENERAL CONSTRUCTION NOTES AND MASTER LEGEND
C-003	DEMOLITION PLAN
C-004	HORIZONTAL CONTROL & SIGNAGE & STRIPING PLAN
C-005	EROSION CONTROL PLAN
C-006	EROSION CONTROL NOTES AND DETAILS
C-007	GRADING PLAN
C-008	DRAINAGE BASIN EXHIBIT
C-009	OVERALL UTILITY PLAN
C-010	SITE & STREET DETAILS
C-011	WATER & SANITARY SEWER DETAILS
C-012	SANITARY SEWER, STORM SEWER, & DRAINAGE DETAILS
D. 1 - (1 - 0)	//

VICINITY MAP

Refer to the "Geotechnical Subsurface Exploration Report, Custom Blending Facility, Fort Collins, CO, Soilogic Project No. 07-1098", dated January 10, 2008, by Soilogic, Inc. for site excavation, grading, and paving recommendations.



FORT COLLINS - LOVELAND WATER DISTRICT **SOUTH FORT COLLINS SANITATION DISTRICT**

r. Michael D. DiTullio, Manager fr. Terry Farrill, P.E., District Engineer

All changes, addendums, additions, deletions and modifications to these drawings must be approved, in writing, by the Fort Collins-Loveland

Water District and the South Fort Collins Sanitation District

THESE PLANS HAVE BEEN REVIEWED BY THE UTILITY PLAN APPROVAL LOCAL ENTITY FOR CONCEPT ONLY. THE REVIEW DOES NOT IMPLY RESPONSIBILITY BY ACCURACY AND CORRECTNESS OF THE Water & Wastewater Utility CHECKED BY: Stormwater Utility REQUIRED. THE REVIEW SHALL NOT BE CHECKED BY:. Parks & Recreation CHECKED BY: Traffic Engineer CONSTRUCTION PHASE. CHECKED BY: Date Environmental Planner

City of Fort Collins, Colorado

COLORADO REGISTRATION No.

THE REVIEWING DEPARTMENT, THE LOCAL ENTITY ENGINEER, OR THE LOCAL ENTITY FOR CALCULATIONS. FURTHERMORE, THE REVIEW DOES NOT IMPLY THAT QUANTITIES OF ITEMS ON THE PLANS ARE THE FINAL QUANTITIES CONSTRUED IN ANY REASON AS ACCEPTANCE OF FINANCIAL RESPONSIBILITY BY THE LOCAL ENTITY FOR ADDITIONAL QUANTITIES OF ITEMS SHOWN THAT MAY BE REQUIRED DURING THE

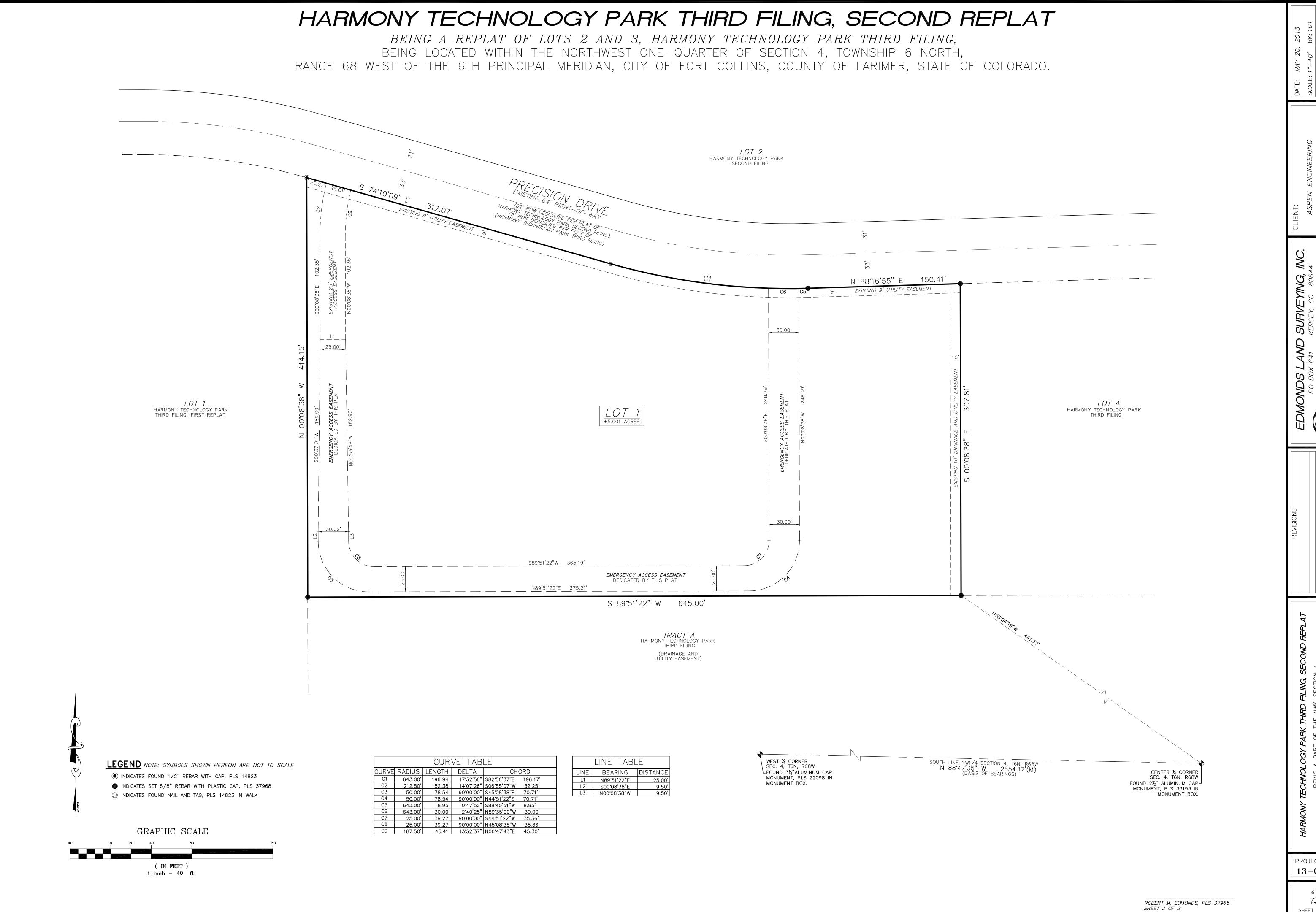
COVI

PROJECT NO: 101-001 06/19/13

SHEET No: C-001 1 OF 12

HEREBY AFFIRM THAT THESE FINAL CONSTRUCTION PLANS WERE PREPARED UNDER MY DIRECT SUPERVISION, IN ACCORDANCE WITH ALL APPLICABLE CITY OF FORT COLLINS AND STATE OF COLORADO STANDARDS AND STATUTES, RESPECTIVELY; AND THAT I AM FULLY RESPONSIBLE FOR THE ACCURACY OF A DESIGN, REVISIONS, AND RECORD CONDITIONS THAT I HAVE NOTED ON THESE PLANS JOHN R. GOOCH, P.E.

PLAT (1-2) (ATTACHED: FOR REFERENCE ONLY)



PROJECT NO.: 13-0386

SHEET 2 OF 2

HARMONY TECHNOLOGY PARK THIRD FILING SECOND REPLAT

graphics site development 6251 greenwood plaza boulevard building 6. suite 100

p 720.488.2626 f 720.488.2625

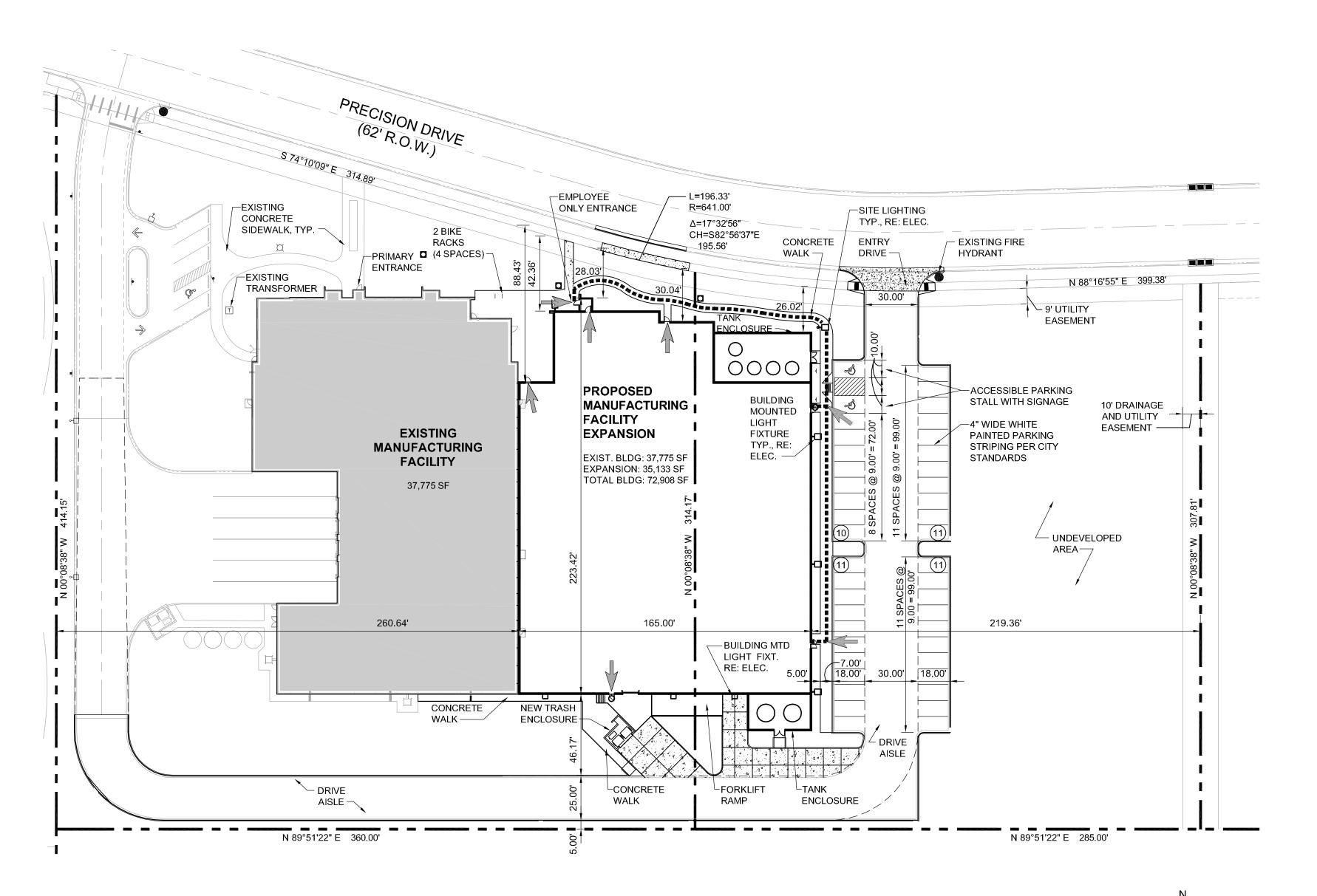
planning

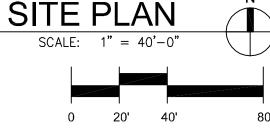
interiors

greenwood village. colorado 80111

WARE MALCOMB Leading Design for Commercial Real Estate

PROJECT DEVELOPMENT PLAN/FINAL DEVELOPMENT PLAN





LEGAL DESCRIPTION:

A TRACT OF LAND, BEING A REPLAT OF LOTS 2 & 3 OF THE HARMONY TECHNOLOGY PARK 3RD FILING, LOCATED IN THE NORTHWEST QUARTER OF SECTION 4, TOWNSHIP 6 NORTH, RANGE 68 WEST OF THE 6TH PRINCIPAL MERIDIAN, CITY OF FORT COLLINS, COUNTY OF LARIMER, STATE OF COLORADO

SITE LEGEND



POLE MOUNTED LIGHT FIXTURE, SEE ELECTRICAL



WALLPACK LIGHT FIXTURE, SEE ELECTRICAL DRAWINGS

FIRE HYDRANT (VERIFY LOCATION WITH CIVIL DRAWINGS)



PARKING STALL COUNT TOTAL.



ON-GRADE BUILDING ENTRY/EXIT

ACCESSIBLE PATH OF TRAVEL

GENERAL NOTES:

- 1. REFER TO UTILITY PLAN FOR LOCATION OF UTILITIES AND DRAINAGE FACILITIES.
- 2. ALL SETBACKS AND LAND USE REQUIREMENTS SHALL CONFIRM TO CITY OF FORT COLLINS, COLORADO ZONING AND LAND USE REGULATIONS IN EFFECT AS OF THE DATE OF APPROVAL OF THIS PROJECT DEVELOPMENT PLAN/FINAL DEVELOPMENT PLAN BY CITY OF FORT COLLINS,
- 3. SIDEWALKS AND RAMPS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE AMERICAN WITH DISABILITIES ACT. HANDICAP ACCESSIBLE ROUTES SHALL SLOPE NO MORE THAN 1:20 IN THE DIRECTION OF TRAVEL AND NO MORE THAN 1:48 CROSS SLOPE.
- 4. SIGNAGE TO COMPLY WITH CITY OF FORT COLLINS SIGN CODE. ALL SIGNS SHALL BE REQUIRED TO APPLY FOR A SIGN PERMIT.
- 5. ALL ROOF-MOUNTED MECHANICAL, ELECTRICAL, OPTICAL AND ELECTRONIC EQUIPMENT SHALL BE SCREENED FROM PUBLIC VIEW.
- 6. PROPOSED GRADES SHALL MATCH OR IMPROVE EXISTING GRADES TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING WHILE PROVIDING A SMOOTH TRANSITION BETWEEN ALL ADJACENT UNDISTURBED GRADES AND PROPOSED GRADES.

		E. HORSETOOTH RD	
S TIMBERLINE RD.	ZIEGLER RD	STRAUSS CABIN RD	
	KECHTER RD.	PRECISION DR	
/IC	CINITY MAP	PROJECT SITE	

STATISTICAL INFORMATION

EXISTING ZONING: HARMONY CORRIDOR (H-C)	ZONE DISTRICT	
GENERAL ZONE LOT INFORMATION	SQUARE FEET	ACRES
LOTS 2 & 3 SIZE (GROSS PROJECT AREA)	217,850.91	5.00
BUILDING COVERAGE	64,102.80 (29.42%); .	29 F.A.R.
DRIVEWAY AND PARKING	52,847.96 (24.26%)	
PUBLIC R.O.W.	0.00 (0.00%)	
OPEN SPACE AND/OR LANDSCAPING	100,900.15 (46.32%)	
LOTS 2 & 3 SIZE (NET PROJECT AREA)	217,850.91	5.00
TOTAL	217,850.91 (100%)	
PRIMARY AND SIDE STREET DESIGNATIONS	HARMONY ROAD AND	PRECISION DRIVE
DESIGN ELEMENTS		
TOTAL FLOOR AREA	64,103 S.F. GROUND	FLOOR = 0.29 F.A.R.
PRIMARY USE	MANUFACTURING	
BUILDING HEIGHT, STORIES (MAX)	6 ALLOWED	2 PROVIDED
BUILDING HEIGHT, FEET	N/A	31'-0" PROVIDED
PARKING	REQUIRED	PROVIDED
STANDARD SPACES (60 EMPLOYEES)	45	45
ACCESSIBLE	2	3
TOTAL	47	48
BICYCLE (FIXED)	4	6 (2 EXIST., 4 NEW)
LOADING SPACES	N/A	5 - LOADING DOCK

OWNER'S CERTIFICATION OF APPROVAL:

THE UNDERSIGNED DOES/DO HEREBY CERTIFY THAT I/WE ARE THE LAWFUL OWNERS OF REAL PROPERTY DESCRIBED ON THIS SITE PLAN AND DO HEREBY CERTIFY THAT I/WE ACCEPT THE CONDITIONS AND RESTRICTIONS SET FORTH ON SAID SITE PLAN.

IN WITNESS WHEREOF, WE HAVE HEREUNTO SET OUR HANDS AND SEALS THIS _____ DAY

(PRINTED NAME)	_	
NOTABIAL CERTIFICATE		

NOTARIAL CERTIFICATE STATE OF COLORADO) COUNTY OF LARIMER)

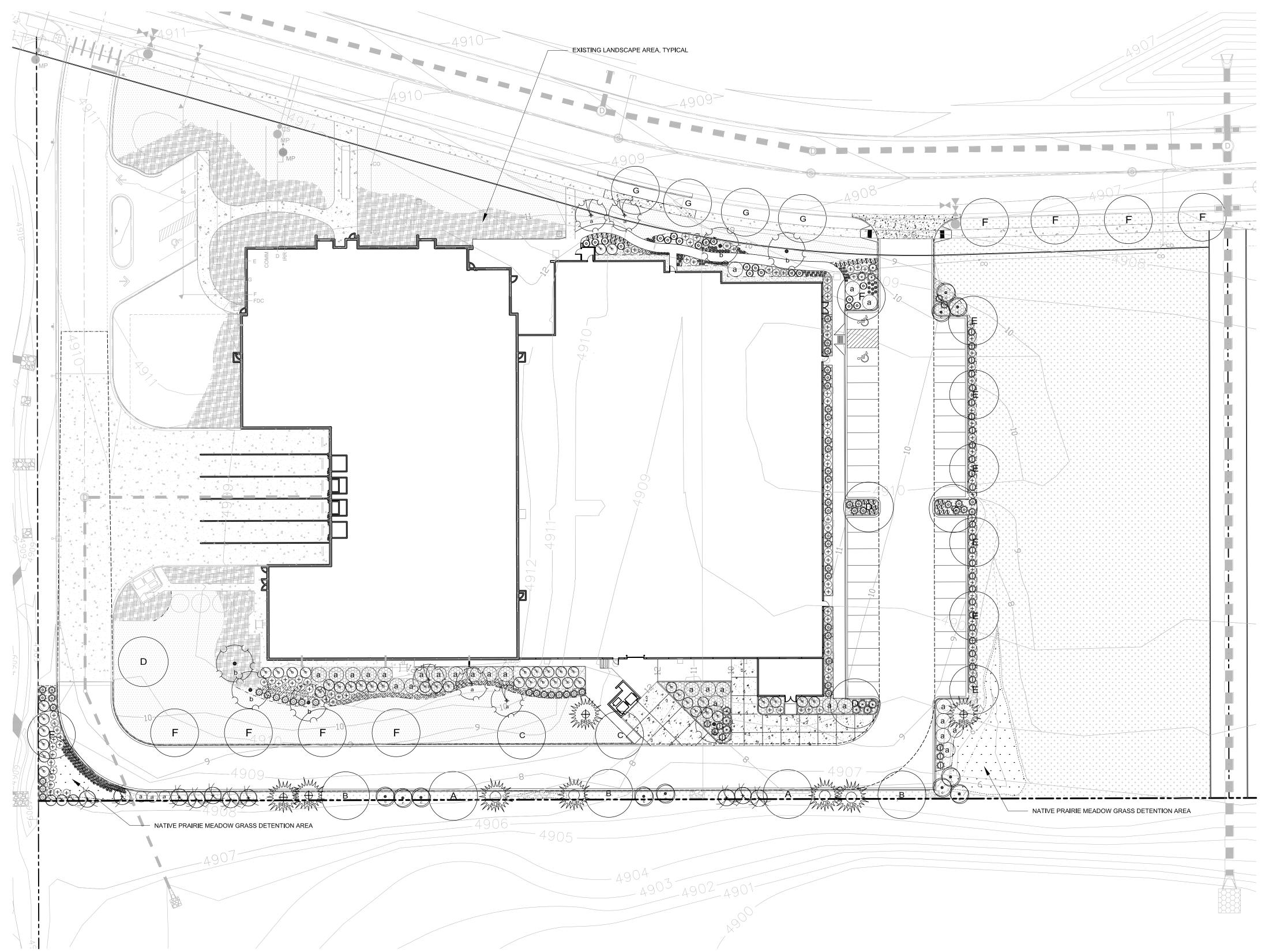
THE FOREGOING INSTRUMENT WAS ACKNOWLEDGED BEFORE ME BY

MY COMMISSION EXPIRES: NOTARY PUBLIC

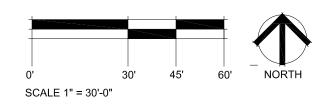
PLANNING APPROVAL:

BY THE DIRECTOR OF COMMUNITY DEVELOPMENT AND NEIGHBORHOOD SERVICES OF THE CITY OF FORT COLLINS, COLORADO THIS _____ DAY OF ____

DIRECTOR OF COMMUNITY DEVELOPMENT AND NEIGHBORHOOD SERVICES



Landscape Plan



Plant List

KEY	QTY	RATIO	COMMON NAME	BOTANICAL NAME	HEIGHT	WIDTH	SIZE	INSTALLATION NOTES
HADE / CANOPY	TREES -	30						
A	2	2.9%	COFFEETREE, KENTUCKY (SEEDLES)	Gymnocladus dioicus 'Espresso'	55'	50'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
В	3	4.3%	GINKGO, AUTUMN GOLD	Ginkgo biloba 'Autumn Gold' (Male only var.)	50'	30'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
C	3	4.3%	HONEYLOCUST, IMPERIAL	Gleditsia triacanthos inermis 'Imperial'	45'	35'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
D	3	4.3%	LINDEN, REDMOND	Tilia americana 'Redmond'	35'	25'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
E	6	8.6%	OAK, BUR	Quercus macrocarpa	50'	50'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
F	9	12.9%	OAK, ENGLISH	Quercus robur	40'	40'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
G	4	5.7%	OAK, SHUMARD RED	Quercus shumardii	50'	50'	2" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
VERGREEN TRE	ES -	8						
William Can	4	5.7%	PINE, AUSTRIAN	Pinus nigra	40'	40'	6'-8' BB	FULL SPECIMEN, EVENLY AND WELL BRANCHED W/ STRAIGHT TRUNK & TOP LEADER
MAN DE STATE OF THE STATE OF TH	4	5.7%	SPRUCE, BAKERI	Picea pungens 'Bakeri'	35'	20'	6'-8' BB	FULL SPECIMEN, EVENLY AND WELL BRANCHED W/ STRAIGHT TRUNK & TOP LEADER
RNAMENTAL TE	REES -	32						
	5	7.1%	CRABAPPLE, SPRING SNOW	Malus spp. 'Spring Snow'	20'	20'	6' Multi- Stem	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
b	6	8.6%	LILAC, JAPANESE LILAC TREE	Syringa reticulata	20'	20'	1.5" cal. BB	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADER
	11	15.7%	MAPLE, TARTARIAN	Acer tataricum	15'	15'	4' Multi- Stem	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADERS
\bigcirc	10	14.3%	OAK, GAMBLE	Quercus gambelii	12'	9'	4' Multi- Stem	BALANCED, WELL BRANCHED W STRAIGHT TRUNK & CENTRAL LEADERS
VERGREEN SHE	RUBS -	100						
Ö	10		JUNIPER, BLUE CHIP	Juniperus horizontalis 'Blue Chip'	1'	6'	5 Gallon	6" (h) FULL SPECIMEN, EVENLY AND WELL BRANCHED
Φ	32	<u>.</u>	JUNIPER, ARCADIA	Juniperus sabina 'Arcadia'	2.5'	5'	5 Gallon	12" (h) FULL SPECIMEN, EVENLY
	20		JUNIPER, CALGARY CARPET	Juniperus sabina 'Calgary	2'	5'	5 Gallon	AND WELL BRANCHED 12" (h) FULL SPECIMEN, EVENLY
Ç.	38	8	PINE, MUGO SLOWMOUND	Carpet' Pinus mugo 'slowmound'	3'	5'	5 Gallon	AND WELL BRANCHED 18" (h) FULL SPECIMEN, EVENLY
	200000			. mas mage slowmound	9	J	o danon	AND WELL BRANCHED
ECIDUOUS SHR	UBS - 102	200	DOGWOOD, ISANTI	Cornus stolonifera 'Insanti'	4'	4'	5 Gallon	24" (h) FULL SPECIMEN, EVENL
o				22 N 20 20 20 20 20 20 20 20 20 20 20 20 20				AND WELL BRANCHED 24" (h) FULL SPECIMEN, EVENL
©	21		LILAC, DWARF KOREAN	Syringa meyeri 'Palibin'	4'	4'	5 Gallon	AND WELL BRANCHED 24" (h) FULL SPECIMEN, EVENL'
	47	-	PLUM, PURPLE LEAF	Prunus x cistena	8'	6'	5 Gallon	AND WELL BRANCHED
а	30		SERVICEBERRY	Amelanchier alnifolia	15'	8'	5 Gallon	24" (h) FULL SPECIMEN, EVENL' AND WELL BRANCHED
ERENNIALS / GF	RASSES -	250						
Φ	36	-	DAY LILY, AUTUMN RED	Hemerocallis spp 'Autumn Red'	3'	2'	1 Gallon	WELL ROOTED AND ESTABLISHED
Ø	74	2	GRASS, FEATHER REED	Calamagrostis acutiflora 'Karl Foerster'	4'	2'	1 Gallon	WELL ROOTED AND ESTABLISHED
•	34		GRASS, HEAVY METAL BLUE SWITCH	Panicum virgatum 'Heavy Metal'	3'	18"	1 Gallon	WELL ROOTED AND ESTABLISHED
•	106		GRASS, RED SWITCH	Panicum virgatum 'Shenandoah'	3'	18"	1 Gallon	WELL ROOTED AND

19,680 S.F.	IRRIGATED TURF DURA-TURF FESCUE BLEND - SOD/SEED
610 S.F.	TAN CRUSHER FINES 5" DEPTH STABILIZED
4,536 S.F.	SMOOTH RIVER COBBLE SHRUB BED AREAS TO RECEIVE MINIMUM 4"- 6" COBBLE OVER WEED BARRIER FABRIC
4,347 S.F.	PIONEER RAINBOW ROCK MINIMUM 4" DEPTH OF 1.5" - 3" ANGULAR STONE WITH BROWN AND GOLD HUES OVER WEED BARRIER FABRIC
40,533 S.F.	LOW-GROW NON-IRRIGATED NATIVE GRASS MIX PAWNEE BUTTE SEED MIX - SEE NOTES OR APPROVED EQUAL
2,357 S.F.	NON-IRRIGATED NATIVE PRAIRIE MEADOW GRASS MIX ARKANSAS VALLEY SEED MIX - SEE NOTES OR APPROVED EQUAL

Annual Hydrozone Table

ZONE	AREA	WATER USE	GALLONS
HIGH	N/A	18 GAL/SF	0 GAL.
MODERATE	18,333 SF	10 GAL/SF	183,330 GAL.
LOW	6,317	3 GAL/SF	18,951 GAL.
VERY LOW	3,390	0 GAL/SF	0 GAL
TOTAL / AVERAGE	28,040 SF	202,281 GAL	7.2 GAL/SF

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SEAL _____

Harmony Technology Park
Third Filing Second Replat,

PDP/Final Plan

Ft Collins, Colorado

PREPARED FOR

CBI REAL ESTATE HOLDINGS, LLC 3461 Precision Drive Fort Collins, CO 80528

PH: 970.449.8154

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CALL UTILITY NOTIFICATION CENTER OF COLORADO



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CITY COMMENTS 06-19-13

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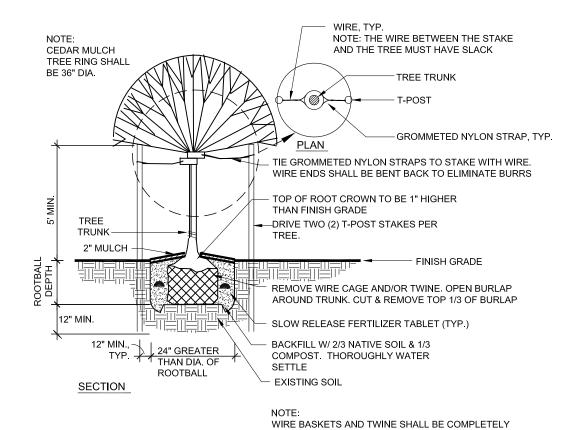
Landscape Plan

SHEET TITLE

SHEET INFORMATION

LS 1

Planting Details



ANCHORS EQUALLY AROUND TRUNK. AVOID DAMAGE TO BRANCHES.

REMOVE WIRE CAGE AND/OR TWINE. OPEN BURLAP AROUND TRUNK. CUT & REMOVE TOP 1/3 OF BURLAP AROUND TRUN

CEDAR MULCH TREE

RING SHALL BE 36" DIA.

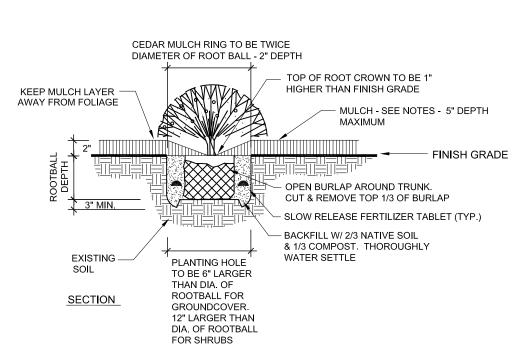
TOP OF ROOT CROWN TO

BE 1" HIGHER THAN

FINISH GRADE

FINISH GRADE

2" MULCH



CONIFER TREE PLANTING DETAIL GROUND COVER & SHRUB PLANTING DETAIL

Landscape Notes

DECIDUOUS TREE PLANTING DETAIL

- 1. LANDSCAPE AREAS SHALL BE MAINTAINED, INCLUDING MOWING, WATERING AND FERTILIZING BY CONTRACTOR, UP TO FINAL ACCEPTANCE. AT SUCH TIME OWNER WILL BE RESPONSIBLE FOR ALL MAINTENANCE. LANDSCAPE AND IRRIGATION WILL BE WARRANTED FOR ONE (1) FULL YEAR AFTER FINAL ACCEPTANCE.
- 2. ANY DISCREPANCIES WITH THE DRAWINGS AND SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE OWNERS REPRESENTATIVE PRIOR TO PROCEEDING WITH CONSTRUCTION.
- 3. CONTRACTOR TO VERIFY ALL FIELD CONDITIONS, EASEMENTS, PROPERTY LINES, ETC. PRIOR TO STARTING WORK. SHOULD ANY DISCREPANCIES, OMISSIONS, OR ERRORS OCCUR, NOTIFY THE OWNER'S REPRESENTATIVE IMMEDIATELY.

REMOVED PRIOR TO TREE INSTALLATION.

- 4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES 48 HRS. PRIOR TO ANY EXCAVATION OR PLANTING.
- 5. LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY COORDINATION WITH SUBCONTRACTORS AS REQUIRED TO ACCOMPLISH PLANTING OPERATIONS
- 6. PER THE CODE REQUIRED TREE UTILITY SEPARATIONS IN 3.2.1 K. STREET AND ORNAMENTAL TREES SHALL BE PLANTED NO CLOSER THAN FORTY (40) FEET AND FIFTEEN (15) FEET RESPECTIVELY FROM STREET LIGHTS. NO TREES SHALL BE PLANTED WITHIN TEN (10) FEET FROM WATER AND SEWER LINES, FOUR (4) FEET FROM GAS, TELEPHONE AND ELECTRIC UTILITIES, SIX (6) FEET FROM STORM SEWER LINES ,SIX (6) FEET FROM WATER AND SEWER SERVICE LINES AND EIGHT (8) FEET FROM ANY DRIVEWAY OR CURBCUT. TREE/UTILITY AND TRAFFIC CONTROL DEVICE SEPARATIONS SHALL NOT BE USED AS A MEANS OF AVOIDING THE PLANTING OF REQUIRED STREET TREES
- 7. MINIMUM CLEARANCE OF THREE (3) FEET ON EACH SIDE OF FIRE DEPARTMENT CONNECTION (FDC). NO VEGETATION OTHER THAN TURF OR GROUND COVERS PLANTED IN FRONT OF FDC.
- 8. ANY OBJECT WITHIN THE SIGHT DISTANCE EASEMENT MORE THAN 30 INCHES ABOVE THE FLOWLINE ELEVATION OF THE ADJACENT STREET SHALL CONSTITUTE A SIGHT OBSTRUCTION, AND SHALL BE REMOVED OR LOWERED. SUCH OBJECTS INCLUDE BUT ARE NOT LIMITED TO BERMS, BUILDINGS, PARKED VEHICLES ON PRIVATE PROPERTY, CUT SLOPES, HEDGES, TREES, BUSHES, UTILITY CABINETS, OR TALL CROPS. MAILBOX CLUSTERS MUST BE INSTALLED A MINIMUM OF 2 FEET FROM BACK OF WALK AND NOT CAUSE ANY SIGHT OBSTRUCTION. IN NO CASE SHALL ANY PERMANENT OBJECT ENCROACH INTO THE LINE-OF-SIGHT OF ANY PART OF THE SIGHT-DISTANCE TRIANGLE. STREET TREES REQUIRED ARE EXCEPTED FROM THIS REQUIREMENT. TREES ARE PERMITTED IF PRUNED UP TO 8 FEET. IN ADDITION TO THE SIGHT DISTANCE TRIANGLE REQUIREMENTS, A CLEAR SPACE ZONE IS REQUIRED WITHIN ALL CURB RETURNS (MEASURED FROM POINT OF CURB TO POINT OF CURB RETURN) WHERE NO TREES, SHRUBS, AESTHETIC STRUCTURES/FEATURES, MONUMENT SIGNS, OR OBJECTS THAT HAVE THE POTENTIAL TO HINDER DRIVER VISIBILITY, AND/OR PEDESTRIAN AND BICYCLE SAFETY, ARE ALLOWED. SEE LARIMER COUNTY URBAN AREA STREET STANDARDS SECTION 7.4.1.C. FOR ADDITIONAL INFORMATION.
- 9. IF TREES OR SHRUBS ARE LOCATED ON TOP OF FIELD VERIFIED UTILITIES, CONTRACTOR SHALL NOTIFY OWNER BEFORE ANY DIGGING HAS COMMENCED. VERIFY WITH OWNER IF AND WHICH SHRUBS/TREES SHALL BE TAKEN OUT OF PROJECT/CONTRACT.
- 10. TO THE MAXIMUM EXTENT FEASIBLE, TOPSOIL THAT IS REMOVED DURING CONSTRUCTION ACTIVITY SHALL BE CONSERVED FOR LATER USE ON AREAS REQUIRING REVEGETATION AND LANDSCAPING.
- 11. EXCAVATED MATERIAL TO BE USED AS FILL WILL HAVE ALL ROCKS, DEBRIS, WASTE MATERIAL, FROZEN MATERIAL, VEGETATION LARGER THAN 3" IN ANY DIMENSION REMOVED BEFORE PLACEMENT AND COMPACTION OF SOIL.
- 12. PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING FOUNDATIONS AND A SMOOTH TRANSITION BETWEEN ALL ADJACENT EXISTING GRADES AND
- 13. PRIOR TO FINE GRADING, DURA-TURF TALL FESCUE SOD OR DURA-TURF TALL FESCUE SEEDED AREAS AND PLANTING BEDS, SHALL BE THOROUGHLY LOOSENED AND TILLED TO A 6" DEPTH. REMOVE ALL UNSUITABLE TOPSOIL, INCLUDING ALL ROCKS LARGER THAN 3 INCHES IN ANY DIRECTION, ALL CONCRETE, TRASH, DEBRIS, WEEDS, ROOTS AND OTHER WASTE MATERIALS.
- 14. AFTER FINE GRADING AND REMOVAL OF UNSUITABLE MATERIALS THE SOIL IN ALL LANDSCAPE AREAS, INCLUDING PARKWAYS AND MEDIANS, SHALL BE THOROUGHLY LOOSENED TO A DEPTH OF NOT LESS THAN EIGHT (8) INCHES AND SOIL AMENDMENT SHALL BE THOROUGHLY INCORPORATED INTO THE SOIL OF ALL LANDSCAPE AREAS TO A DEPTH OF AT LEAST SIX (6) INCHES BY TILLING, DISCING OR OTHER SUITABLE METHOD, AT A RATE OF AT LEAST THREE (3) CUBIC YARDS OF SOIL AMENDMENT PER ONE THOUSAND (1,000) SQUARE FEET OF LANDSCAPE AREA.
- 15. UNIFORMLY COMPACT AND FINE GRADE THESE SOD / GRASS AREAS AND PLANTING BEDS TO A SMOOTH SURFACE, FREE FROM IRREGULAR SURFACE CHANGES. CUT OUT SOFT SPOTS, FILL IN LOW SPOTS AND TRIM HIGH SPOTS TO COMPLY WITH REQUIRED GRADE TOLERANCES.
- 16. ONCE COMPACTED AND FINE GRADED ALL ROCKS, DEBRIS, WASTE MATERIAL AND VEGETATION MATERIAL LARGER THAN 1/2" WILL BE RAKED FROM THE SURFACE AND REMOVED FROM SITE.
- 17. DURA-TURF TALL FESCUE BLEND TO BE 100% COLORADO GROWN SPECIFICALLY FOR LOW WATER LAWN APPLICATIONS WITH MINIMUM THREE (3) IMPROVED VARIETIES, HAVING A HEALTHY VIGOROUS ROOT SYSTEM. ONCE TURF IS LAID IT SHALL BE PROPERLY ROLLED, COMPACTED AND PUSHED TOGETHER TO ELIMINATE ANY GAPS BETWEEN ROLL EDGES. APPLY FERTILIZER IN THESE AREAS PER SOD FARM'S RECOMMENDATION.
- 18. ALL PLANT MATERIALS ARE SIZED AND OUTLINED IN PLANT LIST. ALL PLANTS TO BE PLANTED IN AMENDED SOIL AND STAKED AS SHOWN IN DETAILS. ALL PLANT MATERIAL SHALL MEET OR EXCEED THE CODE OF STANDARDS CURRENTLY RECOMMENDED BY THE COLORADO NURSERY ACT FOR NUMBER ONE GRADE
- 19. IF PLANTS ARE IN NEED OF REPLACEMENT DUE TO DECLINING HEALTH, DISEASE, OR DEATH, THE PLANTS MUST BE REPLACED WITH THE ORIGINAL SPECIES UNLESS APPROVED BY THE CITY.
- 20. CHANGES IN PLANT SPECIES OF PLANT LOCATIONS FROM WHAT IS LISTED ON THE LANDSCAPE PLAN WILL REQUIRE THE APPROVAL OF THE CITY PRIOR TO INSTALLATION OF REPLACEMENT. OVERALL QUANTITY AND QUALITY TO BE CONSISTENT WITH THE APPROVED PLANS. IN THE EVENT OF CONFLICT WITH THE QUANTITIES INCLUDED IN THE PLANT LIST, SPECIES AND QUANTITIES SHALL BE PROVIDED.
- 21. A PERMIT MUST BE OBTAINED FROM THE CITY FORESTER BEFORE ANY TREES OR SHRUBS AS NOTED ON THIS PLAN ARE PLANTED, PRUNED OR REMOVED ON THE PUBLIC RIGHT-OF-WAY. THIS INCLUDES ZONES BETWEEN THE SIDEWALK AND CURB, MEDIANS AND OTHER CITY PROPERTY. THIS PERMIT SHALL APPROVE THE LOCATION AND SPECIES TO BE PLANTED. FAILURE TO OBTAIN THIS PERMIT MAY RESULT IN REPLACING OR RELOCATING TREES AND A HOLD ON CERTIFICATE OF OCCUPANCY.
- 22. ALL TREES AND SHRUBS TO BE BALLED AND BURLAPPED, OR CONTAINERIZED.
- 23. ALL EXISTING TREES TO REMAIN UNLESS OTHERWISE NOTED
- 24. ALL PLANT MATERIAL SHALL HAVE ALL WIRE, TWINE, BASKETS, BURLAP, AND ALL OTHER NON-BIODEGRADABLE CONTAINMENT MATERIAL REMOVED FROM THE TRUNK AND/OR ROOT BALL OF THE PLANT, PRIOR TO PLANTING.
- 25. ALL SHRUB BEDS SHALL HAVE MINIMUM 4" DEPTH SHREDDED CEDAR MULCH NATURAL COLOR AND/OR WASHED SMOOTH COBBLE. A CONTINUOUS LAYER OF TYPAR LANDSCAPE FABRIC OR APPROVED EQUAL SHALL BE INSTALLED IN ALL SHRUB BEDS WITH 6" OVERLAP AT SEAMS WITH 4" STAPLES 4' O.C. IN ALL DIRECTIONS
- 26. EDGING BETWEEN GRASS TYPES AND SHRUB BEDS / ROCK COBBLE SHALL BE HEAVY DUTY STEEL EDGER MIN. 1/8" X 4" WITH MACHINE ROLLED TOP AND SHALL BE SET LEVEL WITH THE TOP OF THE ADJACENT SOD. NO EDGING SHALL BE USED BETWEEN CEDAR MULCH AND COBBLE TRANSITIONS.
- 27. PARKWAY LANDSCAPING INCLUDING TREES SHALL BE MAINTAINED BASED ON CITY CODES AND POLICIES.

Native Grass Seed Mix

NOTE: THE WIRE BETWEEN THE STAKE

GROMMETED NYLON STRAP, TYP

DRIVE THREE (3) T-POSTS PER TREE

DRIVE TWO (2) T-POSTS FOR TREES 6' IN HEIGHT OR LESS. SPACE

FOR TREES OVER 6' IN HEIGHT

AND THE TREE MUST HAVE SLACK

TIE GROMMETED NYLON STRAPS TO STAKE WITH WIRE

WIRE ENDS SHALL BE BENT BACK TO ELIMINATE BURRS

TOP OF ROOT CROWN TO BE 1" HIGHER

THAN FINISH GRADE

NATIVE GRASS - NATIVE PRAIRIE MEADOW GRASS MIX:

- 1. SEED SHALL BE AS MANUFACTURED BY PAWNEE BUTTE SEED, INC OR ARKANSAS VALLEY SEEDS.
- 2. SEED SHALL BE A MIXTURE THAT MATCHES THE FOLLOWING:

COMMON NAME	%	LBS.P.L.S./AC
SIDEOATS GRAMA	17.5%	2.8
BUFFALOGRASS	38.8%	6.2
BLUE GRAMA	3.7%	0.6
INLAND SALTGRASS	5.6%	0.9
BOTTLEBRUSH SQUIRRELTAIL	11.3%	1.8
PRAIRIE JUNEGRASS	1.2%	0.2
WESTERN WHEATGRASS	20.0%	3.2
ALKALI SACATON	1.9%	0.3

- 3. DRILLED APPLICATION RATE: 16.0 LBS (PLS) PER ACRE (0.37 LBS / 1000 SF) IN TWO PERPENDICULAR DIRECTIONS.
- 4. NATIVE SEED AREAS: ADEQUATE TEMPORARY IRRIGATION OR BY WATER TRUCK WILL BE PROVIDED FOR THE ESTABLISHMENT AND MAINTENANCE FOR THESE SEEDED AREAS, AND THAT NATIVE GRASSES SHALL BE MAINTAINED IN A CONDITION OF ACCEPTABLE HEIGHT, FREE OF WEEDS, TRASH AND DEBRIS, AND SHALL NOT REPRESENT A FIRE HAZARD NOR BECOME A NUISANCE SITE FOR WATER OR WIND EROSION.

MULCH IN ALL NATIVE SEED AREAS:

- 1. IMMEDIATELY FOLLOWING THE RAKING OPERATION, ADD STRAW MULCH TO THE SEEDED AREAS.
- APPLY STRAW MULCH AT A MINIMUM OF 1.5 TONS PER ACRE OF AIR DRY MATERIAL. SPREAD STRAW MULCH UNIFORMLY OVER THE AREA WITH MECHANICAL MULCH SPREADER / CRIMPER. DO NOT MULCH WHEN WIND VELOCITY EXCEEDS 10 MPH.
- 3. WHEREVER THE USE OF CRIMPING EQUIPMENT IS PRACTICAL, PLACE MULCH IN THE MANNER NOTED ABOVE AND ANCHOR IT INTO THE SOIL. USE A DISC SUCH AS A MULCH TILLER, WITH A FLAT SERRATED DISC AT LEAS ¼ INCH IN THICKNESS, HAVING DULL EDGES, AND SPACE NO MORE THAN 9 INCHES APART, WITH DISCS OF SUFFICIENT DIAMETER TO PREVENT THE FRAME OF THE EQUIPMENT FROM DRAGGING THE MULCH. ANCHOR MULCH A MINIMUM DEPTH OF 2 INCHES AND ACROSS THE SLOPE WHERE PRACTICAL WITH NO MORE THAN TWO PASSES OF THE ANCHORING EQUIPMENT.
- 4. IMMEDIATELY UPON COMPLETION OF THE MULCHING AND BINDING OPERATION, THE SEEDED AREAS SHALL BE IRRIGATED, KEEPING THE TOP 2 INCHES OF SOIL EVENLY MOIST UNTIL SEED HAS UNIFORMLY GERMINATED AND GROWN TO A HEIGHT OF 2-INCHES.
- 5. WATERING APPLICATION SHALL BE DONE IN A MANNER WHICH WILL PROVIDE UNIFORM COVERAGE BUT WHICH WILL NOT CAUSE EROSION, MOVEMENT, OR DAMAGE TO THE FINISHED SURFACE.

Irrigation Notes

- 1. ENTIRE IRRIGATION SYSTEM AND ASSOCIATED IRRIGATION TAP(S) TO BE SIZED, DESIGNED AND BUILT BY CONTRACTOR. IRRIGATION CONTRACTOR SHALL VERIFY P.S.I. AND GPM AVAILABLE. SYSTEM SHALL BE DESIGNED TO MEET THE AVAILABLE P.S.I. AND GPM. IF NECESSARY CONTACT THE WATER DISTRICT PRIOR TO BEGINNING DESIGN TO OBTAIN AVAILABLE PRESSURES.
- 2. ALL INDICATED SOD GRASS AREAS ARE TO BE IRRIGATED BY A PERMANENT UNDERGROUND AUTOMATIC IRRIGATION SYSTEM. TURF AREAS LESS THAN 25 FEET IN WIDTH ARE TO BE IRRIGATED WITH POP-UP SPRAY HEADS AND AREAS GREATER THAN 25 FEET SHALL USE A ROTOR POP-UP SPRAY SYSTEM.
- 3. ALL TREES, SHRUBS AND PERENNIALS OUTSIDE OF IRRIGATED TURF AREAS, ARE TO BE IRRIGATED WITH A PERMANENT DRIP OR BUBBLER IRRIGATION
- 4. IRRIGATION SYSTEM WITH RAIN SENSOR AND NECESSARY SLEEVING WILL BE DESIGNED AND BUILT BY CONTRACTOR AND ADJUSTED TO A LOW WATER REQUIREMENT. BASED ON THE NEEDS OF SELECTED PLANT MATERIAL.
- 5. QUICK COUPLERS SHALL BE PROVIDED AT EACH POINT OF CONNECTION AND AT REGULAR SPACING ALONG THE IRRIGATION MAINLINE. SPACING OF QUICK COUPLES SHALL NOT EXCEED 200 FEET. LOCATE QUICK COUPLING VALVE AT A POINT OF EASY ACCESS.
- 6. FINAL LOCATION OF IRRIGATION HEADS MUST BE APPROVED BY OWNER'S REPRESENTATIVE PRIOR TO PLANTING. HEAD LOCATION SHALL BE COORDINATED IN
- THE FIELD WITH EXISTING SITE CONDITIONS AND PLANT MATERIAL.

 7. ALL IRRIGATION TRENCHES SHALL BE PROPERLY WATERED AND COMPACTED TO AVOID FUTURE SETTLING. ANY SETTLING DURING WARRANTY PERIOD WILL
- 8. COORDINATE ALL IRRIGATION WORK WITH EXISTING UTILITIES AND RESPECTIVE TRADES.

BE REPAIRED BY THE CONTRACTOR AT NO COST TO THE OWNER.

9. ALL IRRIGATION SLEEVING SHALL BE PROVIDED AND INSTALLED BY GENERAL CONTRACTOR. IRRIGATION CONTRACTOR SHALL COORDINATE SLEEVING LOCATIONS WITH GENERAL CONTRACTOR. ALL IRRIGATION SLEEVING TO BE STAKED IN THE FIELD OR LOCATED ON DIMENSIONED "AS-BUILT" DRAWING BY THE GENERAL CONTRACTOR TO ALLOW FUTURE USE AND LOCATION.

General Notes:

- 1. ALL SIGNS SHALL BE REQUIRED TO APPLY FOR SIGN PERMIT.
- 2. PROPOSED GRADES SHALL MATCH OR IMPROVE EXISTING GRADES TO PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING WHILE PROVIDING A SMOOTH TRANSITION BETWEEN ALL ADJACENT UNDISTURBED GRADES AND PROPOSED GRADES.
- 3. JOB SITE TO BE KEPT CLEAN AT ALL TIMES AND CONSTRUCTION AREAS ARE TO BE MAINTAINED FOR SAFETY.
- 4. SOILS DISTURBED ADJACENT TO WORK AREA, INCLUDING AREAS OUTSIDE OF CONSTRUCTION LIMITS, DUE TO NEW CONSTRUCTION ARE TO BE REGRADED AND SURFACE CONDITIONS REPAIRED EQUIVALENT TO THAT CONDITION PRIOR TO START OF WORK.
- 5. PROTECT EXISTING SURFACES AND SOILS, BOTH INSIDE AND OUTSIDE OF CONSTRUCTION LIMITS, DURING CONSTRUCTION. IF GRADES, CONCRETE OR ASPHALT ARE DAMAGED DUE TO CONSTRUCTION OPERATIONS OR WEATHER THE CONTRACTOR IS RESPONSIBLE FOR REPAIR TO THAT EQUIVALENT TO EXISTING CONDITIONS AT NO EXPENSE TO THE OWNER / CITY.
- 6. CONTRACTOR IS RESPONSIBLE FOR SETUP OF BARRICADES, WARNING SIGNAGE, OR OTHER PROTECTIVE DEVICES IF ANY EXCAVATIONS ARE LEFT EXPOSED AFTER ON-SITE WORK HOURS.
- 7. THE CONTRACTOR SHALL NOT PURPOSEFULLY PROCEED WITH ANY CONSTRUCTION PER PLANS PROVIDED WHEN OBSTRUCTIONS AND/OR GRADE DIFFERENCES EXIST THAT WERE NOT CONSIDERED OR CHANGED AFTER PLANS WERE SUBMITTED. CONTRACTOR SHALL NOTIFY OWNER OR OWNER'S REPRESENTATIVE AND THE CITY OF LOVELAND IF SITUATION ARISES AND REVISIONS ARE NECESSARY.
- 8. THE CONTRACTOR SHALL PREVENT SEDIMENT, DEBRIS AND OTHER POLLUTANTS FROM ENTERING ANY STORM WATER SEWER SYSTEM OR, ADJACENT WATER WAYS, ETC., DURING THE DEMOLITION OR CONSTRUCTION OPERATIONS THAT ARE PART OF THIS PROJECT. THE CONTRACTOR SHALL BE HELD RESPONSIBLE AND EXPENSE FOR THE CORRECTION OF ANY ADVERSE IMPACTS TO THE STORM WATER SEWER SYSTEM OR, ADJACENT WATER WAYS, WETLANDS ETC., RESULTING FROM THE WORK DONE AS PART OF THIS PROJECT/CONTRACT.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE PRIOR TO BIDDING AND CONSTRUCTION, OF BECOMING AWARE OF ALL EXISTING AND PROPOSED UTILITIES, PIPES, STRUCTURES, ETC. CALL UNCC THREE DAYS BEFORE SCHEDULED WORK AT 811 OR 1-800-922-1987.



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SEAL _____

PROJECT TITLE

Harmony Technology Park

Third Filing Second Replat,

PDP/Final Plan

Ft Collins, Colorado

PREPARED FOR

CBI REAL ESTATE HOLDINGS, LLC 3461 Precision Drive Fort Collins, CO 80528

PH: 970.449.8154

NOT FOR CONSTRUCTION FOR REVIEW ONLY



CALL UTILITY NOTIFICATION CENTER OF

CITY COMMENTS 06-19-13

REVISIONS DATE

05-22-13

DATE....

Landscape Notes

SHEET INFORMATION

I S 2

SHEET TITLE

HARMONY TECHNOLOGY PARK THIRD FILING SECOND REPLAT

site development

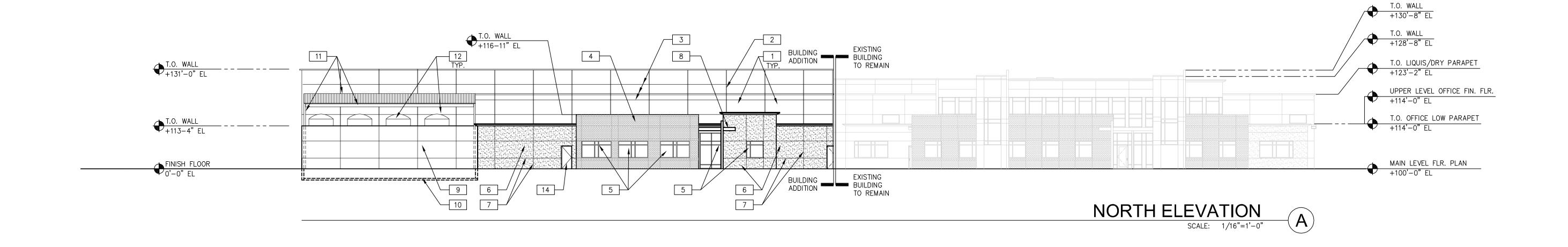
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building 6. suite 100
greenwood village. colorado 80111
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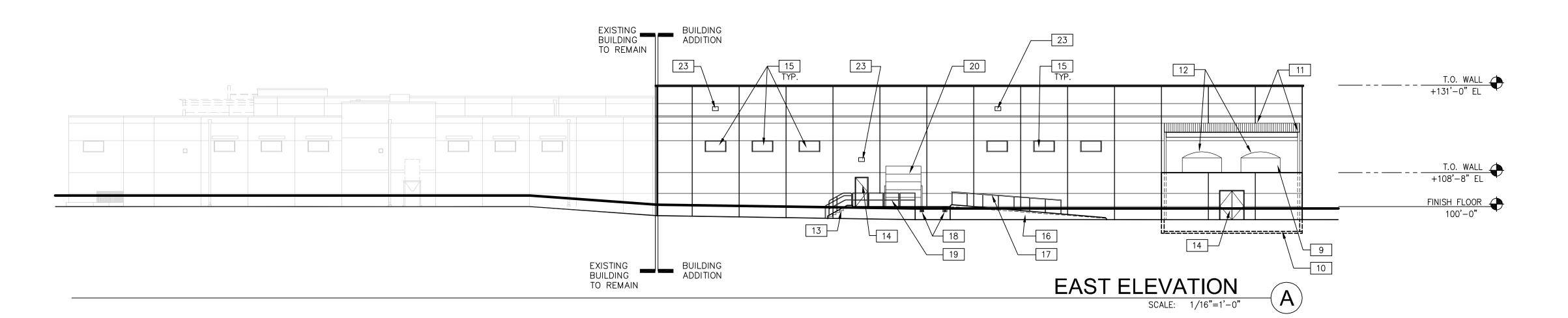
planning

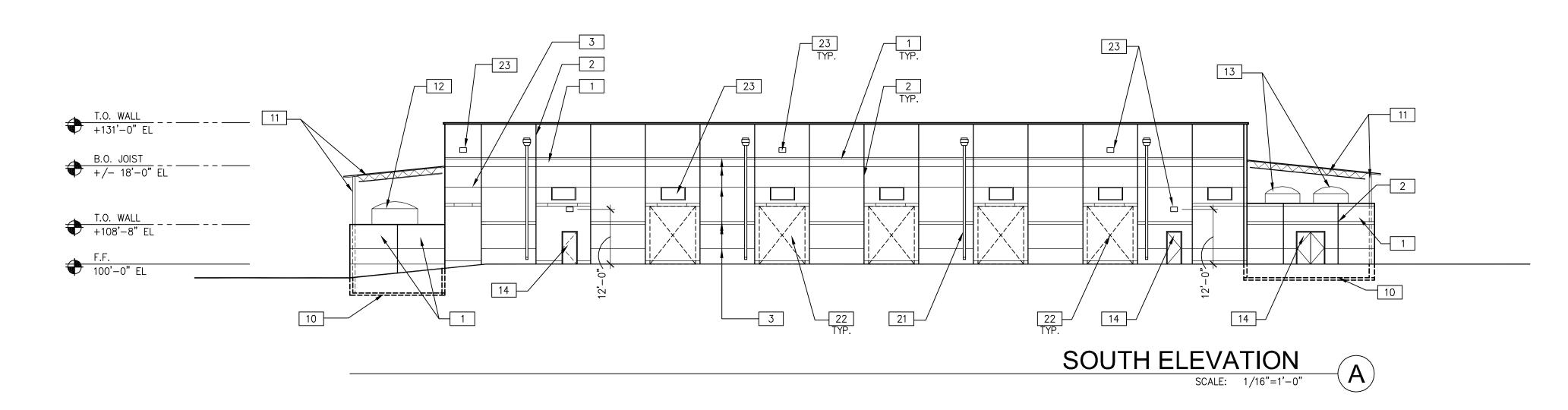
interiors graphics

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PROJECT DEVELOPMENT PLAN/FINAL DEVELOPMENT PLAN







ELEVATION NOTES

ALL PAINT COLORS & EXTERIOR MATERIALS TO MATCH EXISTING FACILIT

- 1 EXTERIOR INSULATED PRECAST CONCRETE WALL PANEL (PAINTED)
- 2 PRECAST CONCRETE PANEL PANEL JOIN
- 3 PRECAST CONCRETE PANEL 1 1/2 " REVEAL
- 4 EXTERIOR BRICK VENEER RUNNING BOND COLOR & BRICK TYPE TO MATCH EXISTING
- 5 ALUMINUM STOREFRONT SYSTEM WITH 1" THICK INSULATED GLAZING.
- 6 3 COAT STUCCO PAINTED TO MATCH EXISTING BUILDING STUCCO FINISH
- 7 STUCCO REVEAL
- 8 PAINTED STEEL TUBE CANOPY TO MATCH EXISTING BUILDING CANOPY SYSTEM
- 9 PROCESS TANK ENCLOSURE PAINTED PRECAST CONCRETE PANELS TO MATCH BUILDING PANELS.
- 10 POURED IN PLACE CONCRETE PIT ENCLOSURE
- 11 TANK ENCLOSURE ROOF STRUCTURE PAINTED STEEL FRAME WITH CORRUGATED METAL DECKING
- 12 PROCESS TANK
- 13 POURED IN PLACE CONCRETE ACCESS STAIR
- 14 HOLLOW METAL DOOR & FRAME PAINTED
- 15 CLEAR ANODIZED STOREFRONT CLERESTORY WINDOW
- 16 POURED IN PLACE CONCRETE RAMP PAINT EXTERIOR FACE TO MATCH BUILDING FIELD COLOR
- 17 PAINTED STEEL GUARD RAIL
- 18 EXTERIOR RUBBER TRUCK BUMPER
- 19 PAINTED STEEL GUARD RAIL REMOVEABLE
- 20 FACTORY FINISHED SECTIONAL OVERHEAD DOOR
- 21 10" X 10" OPEN FACED OVERFLOW DOWNSPOUT WITH 8" RD. DOWNSPOUT PAINTED TO MATCH BUILDING
- 22 KNOCK OUT PANEL FOR FUTURE OPENINGS
- 23 LIGHT FIXTURE RE: PHOTOMETRIC

DELICH ASSOCIATES Traffic & Transportation Engineering

2272 Glen Haven Drive Phone: (970) 669-2061

Loveland, Colorado 80538 Fax: (970) 669-5034



MEMORANDUM

TO:

Dan Berlin, Custom Blending John Gooch, Aspen Engineering Jeff Reed, Agora West Real Estate

City of Fort Collins

FROM:

Matt Delich

DATE:

June 4, 2013

SUBJECT:

Custom Blending Expansion Transportation Impact Study

(File: 1345ME01)

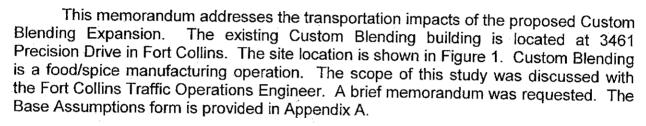


Figure 2 shows the current geometry at the key intersections in the area. There are sidewalks along both sides of Lady Moon Drive between Rock Creek Drive and Precision Drive. There are sidewalks along both sides of Rock Creek Drive between Technology Parkway and Lady Moon Drive. There are also sidewalks along the south side of Precision Drive between the existing Custom Blending site and Lady Moon Drive. There are bicycle lanes along Lady Moon Drive, Rock Creek Drive, and the existing short segment of Technology Parkway.

Figure 3 shows recent peak hour counts at the Ziegler/Rock Creek, Rock Creek/Technology, Rock Creek/Lady Moon, and Lady Moon/Precision intersections. City staff agreed that no new traffic counts would be required. Raw traffic data is provided in Appendix B. Since the traffic counts were obtained on different days, the volumes at the intersections were adjusted/balanced and are shown in Figure 4. Table 1 shows the current morning and afternoon peak hour operation of the key intersections. Calculation forms are provided in Appendix C. A description of level of service for signalized and unsignalized intersections from the 2010 Highway Capacity Manual and a table showing the Fort Collins Motor Vehicle LOS Standards (Intersections) are also provided in Appendix C. The key intersections operate acceptably during the peak hours with existing control and geometry.

Figure 5 shows the site plan for the Custom Blending Expansion. The Custom Blending Expansion will be approximately 38,000 square feet. Access to the expansion site will be primarily via a full-movement access to/from Precision Drive. Trip

Generation, 9th Edition, ITE was used as the reference document in calculating the trip generation. Land use code 130, Industrial Park was used for the Custom Blending Expansion. Table 2 shows the trip generation for the Custom Blending Expansion. The Custom Blending Expansion is expected to generate 260 daily trip ends, 31 morning peak hour trip ends, and 32 afternoon peak hour trip ends.

Directional distribution of the generated trips was determined for the Custom Blending Expansion site and is shown in Figure 6. Figure 7 shows the site generated peak hour traffic assignment of the Custom Blending Expansion.

Figure 8 shows the short range (2018) background morning and afternoon peak hour traffic at the key intersections. Background traffic volume forecasts for the short range (2018) future were obtained by reviewing traffic studies for other developments in this area and reviewing historic counts in the area. Traffic volumes from the Banner Health Medical Campus and 5043 Technology Parkway were used in the traffic forecasts. Table 3 shows the short range (2018) background morning and afternoon peak hour operation at the key intersections. Calculation forms are provided in Appendix D. The key intersections will operate acceptably with the existing control and geometry in the short range (2018) future.

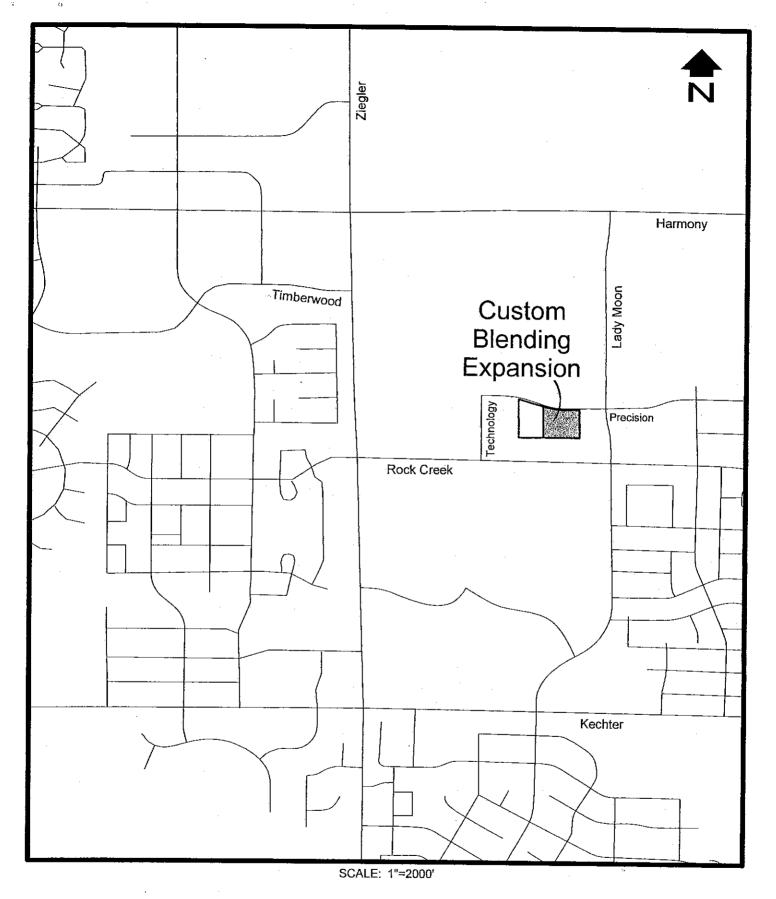
Figure 9 shows the short range (2018) total morning and afternoon peak hour traffic at the key intersections. Table 4 shows the short range (2018) total morning and afternoon peak hour operation at the key intersections. Calculation forms are provided in Appendix E. The key intersections will operate acceptably during the morning and afternoon peak hours with the existing control and geometry.

The Custom Blending Expansion site is in an area within which the City requires pedestrian and bicycle level of service evaluations. Appendix F shows a map of the area that is within 1320 feet of the Custom Blending Expansion site. The Custom Blending Expansion site is located within an area termed as "other," which sets the pedestrian level of service threshold at LOS C for all measured categories. There are two destination areas within 1320 feet of the proposed Custom Blending Expansion: 1) the residential neighborhood to the southeast and 2) the residential apartments to the east. Appendix F contains a Pedestrian LOS Worksheet.

Based upon Fort Collins bicycle LOS criteria, there are no destination areas within 1320 feet of the Custom Blending Expansion site.

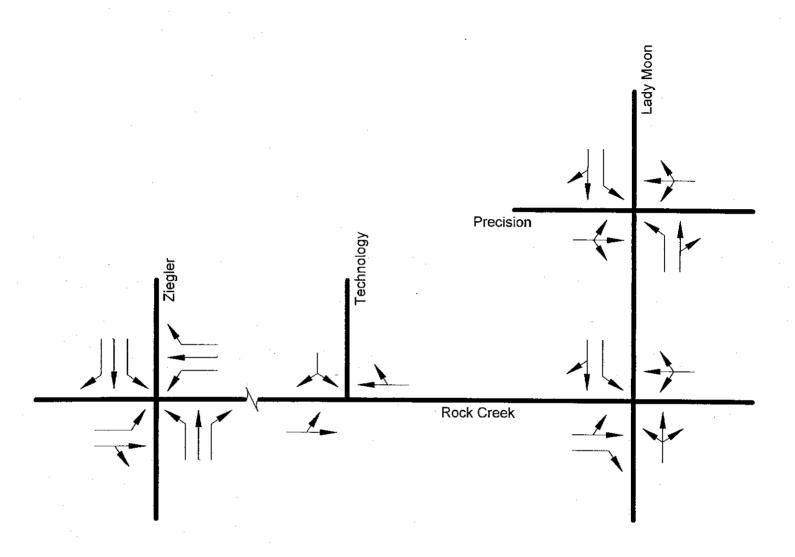
Currently, this area is served by Transfort Routes 16 and 17. The transit service is acceptable.

It is concluded that, with full development of the Custom Blending Expansion, the future level of service at the key intersections will be acceptable. The Custom Blending Expansion can be built without additional geometry or other street improvements.



SITE LOCATION



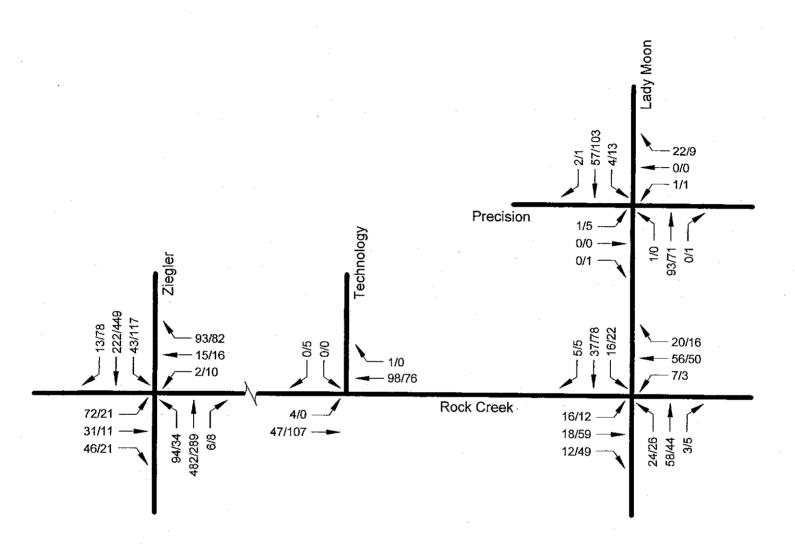


- Denotes Lane

CURRENT INTERSECTION GEOMETRY



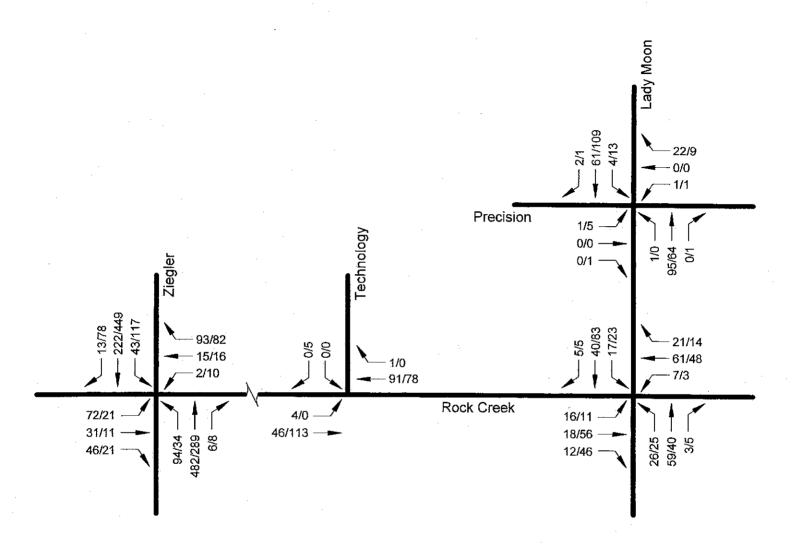




→ AM/PM





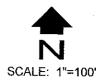


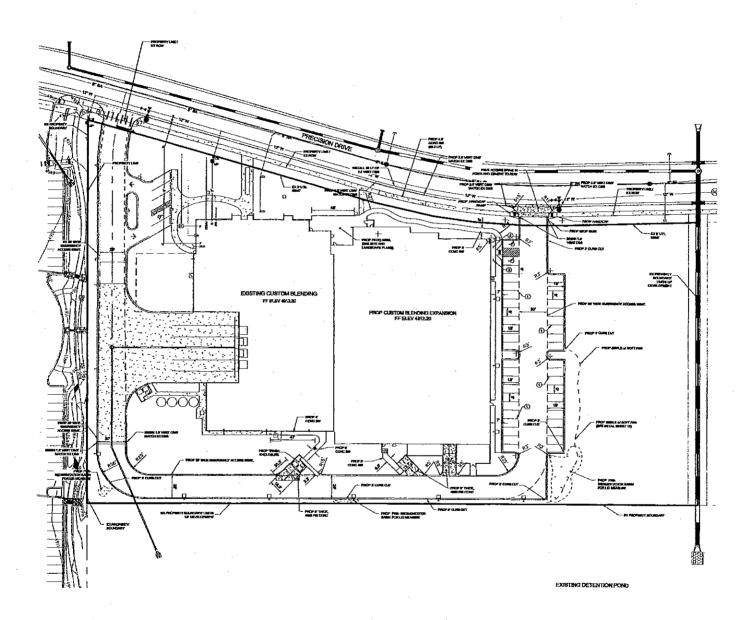
→ AM/PM

ADJUSTED/BALANCED RECENT PEAK HOUR TRAFFIC



TABLE 1 Current Peak Hour Operation								
Intersection	Wayamant	Level of	Service					
	HUTCHE	AM	PN					
	EB LT/T/RT	В	В					
	EB APPROACH	Α	Α					
	WB LT	В	В					
	WBT	В	В					
	WB RT	В	В					
	WB APPROACH	В	В					
Zin edan/Dank Charle	NB LT	Novement Level of Service AM	Α					
Ziegler/Rock Creek (signal)	NB T	В	В					
(Signal)	Novement	Α						
	NB APPROACH	В	В					
·	SB LT	Α	Α					
	SBT	В	В					
	SBRT	Α	A					
	SB APPROACH	Α	В					
	OVERALL	В	В					
Rock Creek/Technology	SB LT/RT	A	Α					
(stop sign)	EB LT/T	Α	Α					
	EB LT/T	A	Α					
	EB LT/T/RT B							
	EB APPROACH	Α	Α					
Deals One still adv Mass	WB LT/T/RT	Α	Α					
Rock Creek/Lady Moon (all-way stop)	NB LT/T/RT	Α	Α					
(all-way stop)	SB LT	Α	Α					
•	SB T/RT	Α	Α					
	SB APPROACH	A	А					
	OVERALL	Α	Α					
	EB LT/T/RT	В	В					
Lady Moon/Precision	WB LT/T/RT	Α	Α					
(stop sign)	NB LT	Α	Α					
	SB LT	А	Α					

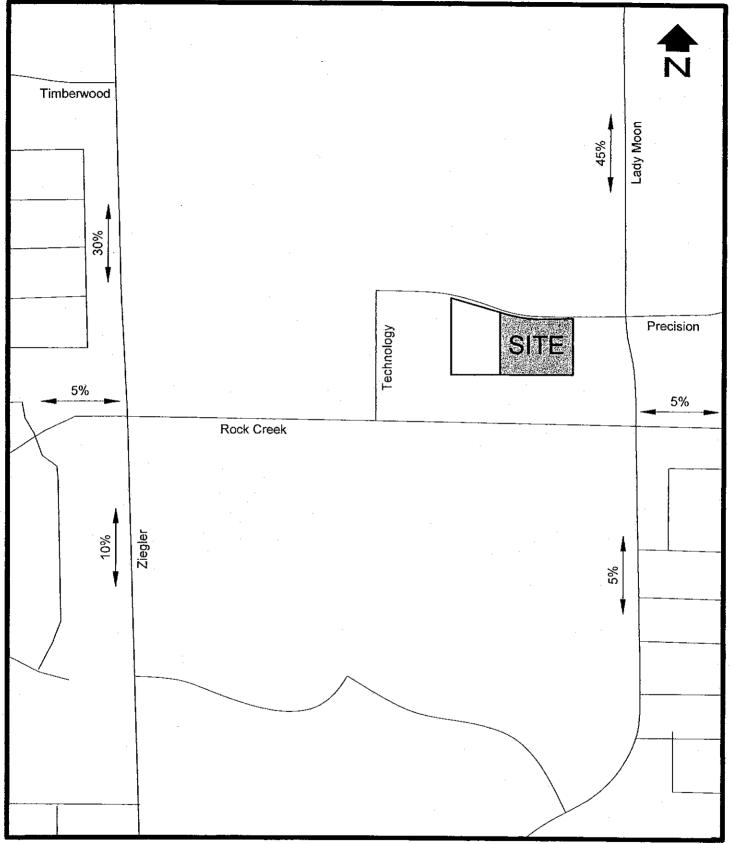




SITE PLAN



	TABLE 2 Trip Generation											
Code	Use	Size	000 000 000 000 000 000 000 000 000 00	DTE Tripa :	3 No. of the	TAX TOTAL CONTRACT	NAME OF PERSONS ASSOCIATED		P Rate			- 6 75 - 11
130	Industrial Park	38.0 KSF	6.83	260	0.67	25	0.15	6	0.18	7	0.67	25

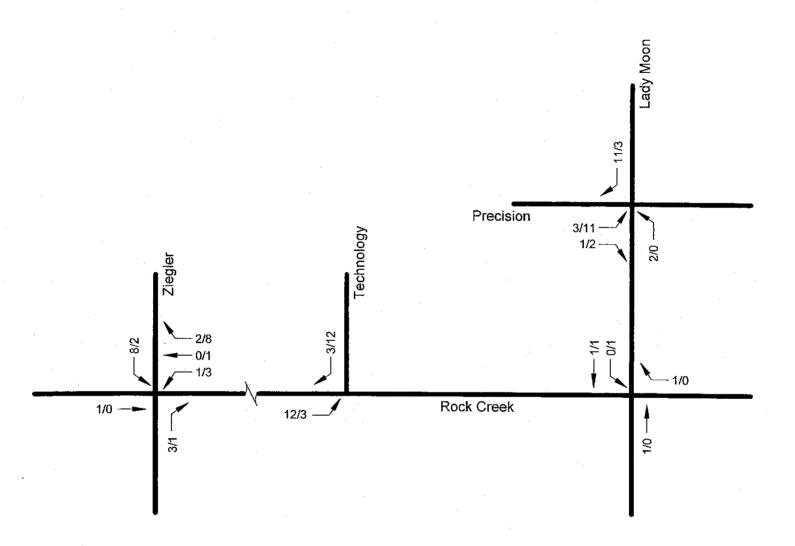


SCALE: 1"=2000'

TRIP DISTRIBUTION





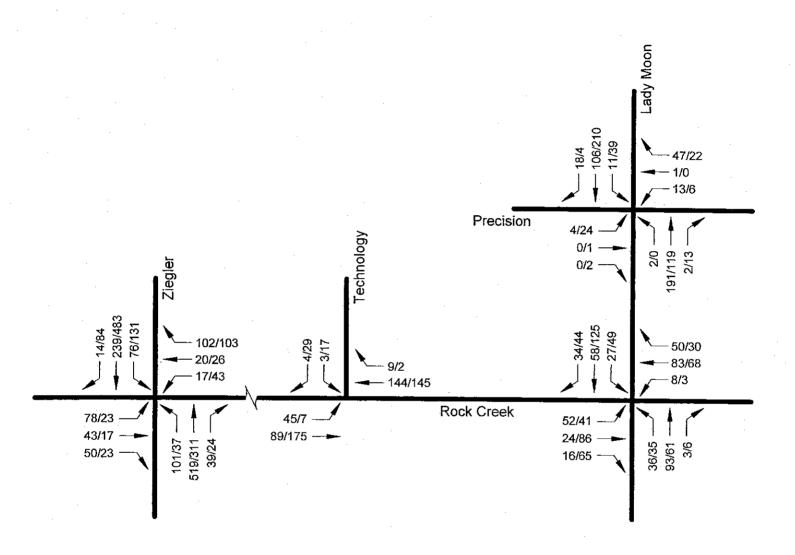


→ AM/PM

SITE GENERATED PEAK HOUR TRAFFIC







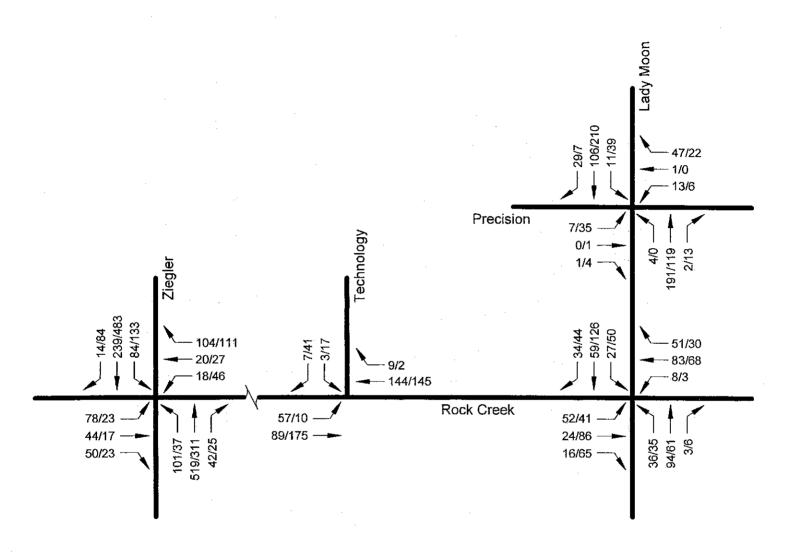
AM/PM

SHORT RANGE (2018) BACKGROUND PEAK HOUR TRAFFIC



TABLE 3 Short Range (2018) Background Peak Hour Operation				
Intersection	Movement	Level o	Service	
	Movement	AM	PM	
	EB LT/T/RT	В	В	
	EB APPROACH	Α	Α	
	WB LT	В	В	
	WBT	В	В	
	WB RT	В	В	
	WB APPROACH	В	В	
Zia glav/Dank Crank	NB LT	А	Α	
Ziegler/Rock Creek (signal)	NB T	В	В	
(Signal)	NB RT	Α	Α	
	NB APPROACH	В	В	
	SB LT	A	A	
	SBT	В	В	
	SB RT	A	A	
	SB APPROACH	A	В	
	OVERALL	В	В	
Rock Creek/Technology (stop sign)	SB LT/RT	В	В	
	EB LT/T	Α	A	
Rock Creek/Lady Moon (all-way stop)	EB LT/T	Α	В	
	EB RT	. A	А	
	EB APPROACH	Α	Α	
	WB LT/T/RT	Α	В	
	NB LT/T/RT	В	В	
	SB LT	A	Α	
	SB T/RT	Α	В	
	SB APPROACH	A	В	
	OVERALL	Α	В	
Lady Moon/Precision (stop sign)	EB LT/T/RT	В	В	
	WB LT/T/RT	В	В	
	NB LT	A	Α	
	SB LT	Α	Α	
	^		L	





→ AM/PM

SHORT RANGE (2018) TOTAL PEAK HOUR TRAFFIC



TABLE 4 Short Range (2018) Total Peak Hour Operation				
Intersection	Movement		f Service	
		AM	PM	
	EB LT/T/RT	В	В	
·	EB APPROACH	A	Α	
	WB LT	В	В	
	WBT	В	В	
·	WB RT	В	В	
	WB APPROACH	В	В	
Ziegler/Rock Creek	NB LT	A	Α	
(signal)	NB T	В	В	
Cognary	NB RT	Α	Α	
	NB APPROACH	В	В	
	SB LT	А	Α	
	SBT	В	В	
·	SB RT	Α	Α	
	SB APPROACH	Α	В	
	OVERALL	В	В	
Rock Creek/Technology (stop sign)	SB LT/RT	A	В	
	EB LT/T	А	Α -	
Rock Creek/Lady Moon (all-way stop)	EB LT/T	Α	В	
	EB RT	A.	Α	
	EB APPROACH	Α	Α	
	WB LT/T/RT	В	В	
	NB LT/T/RT	В	В	
	SB LT	А	Α	
	SB T/RT	Α	В	
	SB APPROACH	Α	В	
	OVERALL	Α	В	
Lady Moon/Precision (stop sign)	EB LT/T/RT	В	В	
	WB LT/T/RT	В	В	
	NB LT	Α	Α	
	SB LT	Α	Α	

APPENDIX A

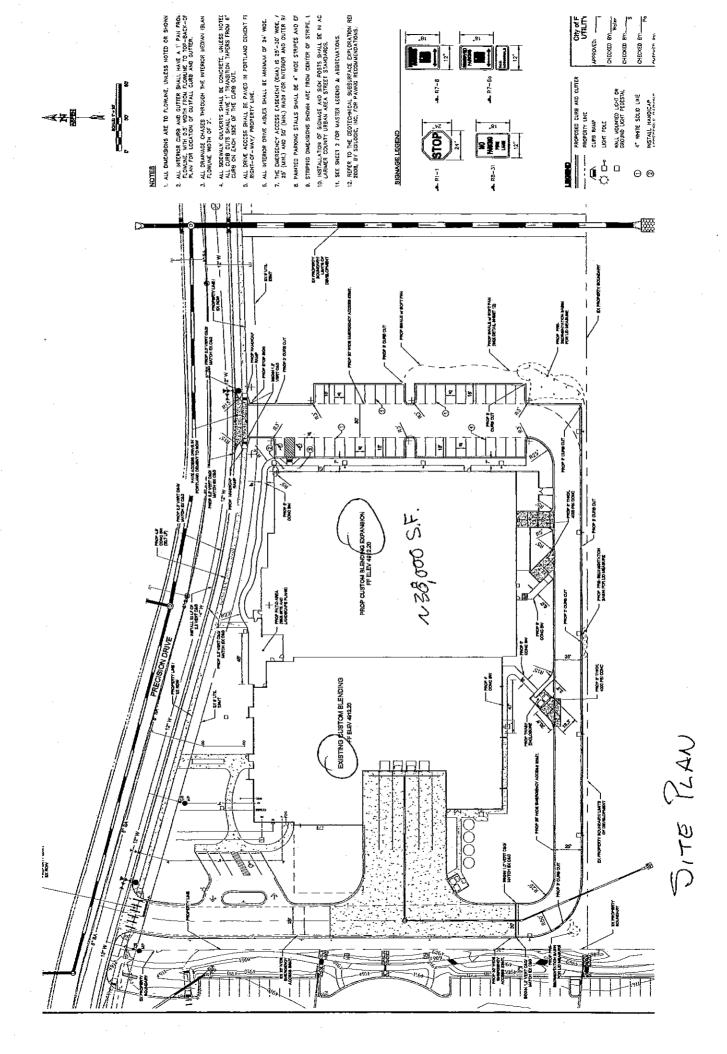
Attachment A Transportation Impact Study Base Assumptions

Project Name Custom BLE	NDING EXPANSION	,	
Project Location NW QUADE	ANT OF ROCK CREEK	LABY MOON	
TIS Assumptions			
Type of Study	Full: NO	Intermediate: USUO	
Study Area Boundaries	North: PRECISION	South: ROCK CEBEL	
	East: LABY MOON	West: ZIEGLER	
Study Years	Short Range: 2018	Long Range: N/A	
Future Traffic Growth Rate	1.5% PLUS KNOWN PRI	OUGCTS*	
Study Intersections		ZIEGLER/ROCKCRE	
·	2.4109 How/PEGOSION 6.		
	3. LANG MOON/ROCK CREEKY.		
	4. POCK CREEK / TECH PRINGS.		
Time Period for Study	AM: 7:00-9:00 PM: 4:00-6:00 Sat Noon; NO		
Trip Generation Rates	PER T.G., 9760	(ATTACHES)	
Trip Adjustment Factors	Passby: 1/A	Captive Market: U/A	
Overall Trip Distribution	SEE ATTACHED SKETCH		
Mode Split Assumptions	N/A		
Committed Roadway Improvements	NOT AWARE OF ANY		
Other Traffic Studies	BANDEL HOSPITAL	•	
Areas Requiring Special Study	USS AVAILABLE COUNTS	- No New Courts	

Date: MAY 24, 2013
Traffic Engineer: DELICH ASSOCIATES
Local Entity Engineer: 15/30/13
1345 BAF



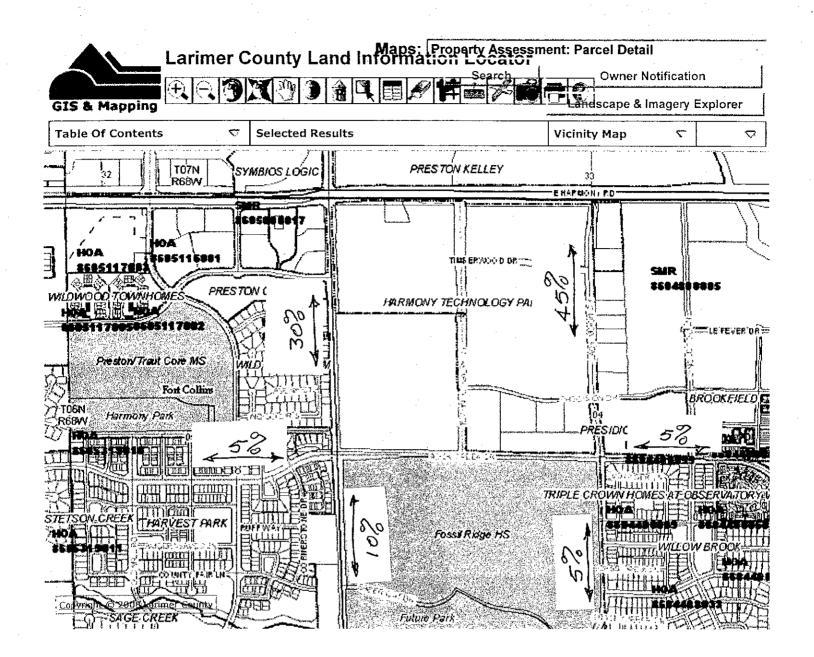
SITE LOCATION



TRIP GENERATION (T.G., 9Th) BUILDING SIZE - ~ 38,000 S.F. INDUSTRIAL PARK (CODE 130) DAILY - (6.83) - 260 AM (0.82) - 31 1N-(828) - 25 OUT-(188) - 6

PM (0.85) - 32

1N.(218)-7 OUT (198)-25



TRIP DISTRIBUTION

APPENDIX B

DELICH ASSOCIATES 2272 GLEN HAVEN DRIVE LOVELAND, CO 80538 Phone: (970) 669-2061

TABULAR SUMMARY OF VEHICLE COUNTS

Date: 3-27-13

Observer: City of Fort Collins

Day: Wednesday

Jurisdiction: Fort Collins

Intersection:

Ziegler/Rock Creek

right turr	otroich.
П	ı
œ	O

Total	All	334	307	275	193
Total	east/west	120	43	43	43
Westbound: Rock Creek	Total	38	34 Maria	29	22
nd:	R	23	20	26	24
stbou	S	8	₹	3	က
We	L	2	0	0	0
Rock Creek	Total	18 18 18 18 18 18 18 18 18 18 18 18 18 1	27 T	# * * *	9 ,
:pu	œ	28	10	ဖ	2
Eastbound:	ဟ	14	9	4	7
Ea		45	ပ္	4	7
Total	north/south	214	264	232	150
Southbound: Ziegler	Total	1000円では、1000円	105	26.	46
nd:	œ	4	2	2	5
thbou	S	58	95	12 42	27
Sou		6	8	12	14 27
Northbound: Ziegler	Total	9	9	82,	104
nd:	α	2	2	-	-
thbou	S 2	25 116	13 144	135	87
Š		25	13	40 135	16
Time	Begins	7:30	7:45	8:00	8:15

100		
249		
110	0.83	
66	0.89	
2	0.47	
2	0.25	
139	0.4	
46	0.41	
31	0.55	
62	0.34	
860		
278	0.66	
•	0.65	
22	95'0	
£3	72.0	
582	0.83	
•	0.75	
482	0.84	
76	0.59	
7:30-8:30	PHF	

		11.5	; -
267	280	262	327
45	35	41	40
35	24	22	27
26	20	16	20
4	2	2	5
ည	5	1	2
	11 Sept.	49	13
4	2	8	2
1	3	4	3
5	9	7	3
222	245	221	287
131	140		210
11 11 12 22	23 149	14 1111154	30 2.00
131	140		210
11 131	23 149	14 1111154	30 2.00
97 11 52	103 23 149	112 14 1154	137 30 210
97 11 52	103 23 149	112 14 1154	137 30 210
23 97 11 115.	103 23 149	au 67	137 30 210
3 F 23 97 11 1454 8	1 23 103 23 149	3 3 7 5 28 112 14 154 154	1 270 30 270 1

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161	
108	22.0
82	0.79
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975	
644	0.77
18	0.65
9	0.82 0
i.	99.0
331	0.86
00	0.89 0.67
289	0.89
34 289	0.61 0.89

DELICH ASSOCIATES 2272 GLEN HAVEN DRIVE LOVELAND, CO 80538 Phone: (970) 669-2061

TABULAR SUMMARY OF VEHICLE COUNTS

Date: 11-28-12

Observer: Joe

Jurisdiction: Fort Collins

Day: Wednesday

Intersection:

Lady Moon/Rock Creek

R = right turn S = straight L = left turn

Total	All	62	75	70	48
Total	east/west	38	38	27	26
Westbound: Rock Creek	Total	26	26	8	•
ind:	œ	9	9	9	2
stbol	တ	19	15	12	10 2
We		-	5	0	*
Eastbound: Rock Creek	Total	24 4 温度	E 7. []	6 1	图 SP
:pur	2	0	3	9	3
stbou	ဟ	∞	4	Į	9
ш.		4	ç	2	2
Total	north/south	41	37	43	22
Southbound: Lady Moon	Total	9			
:pur	а	1	2	2	0
ıthbo(S	10	11	7	5
Sou		5	2	7	2
Northbound: Lady Moon	Total		22	23	175
ipur:	œ	_		-	0
rthbor	ω «	15	15	17	=
Š		თ	9	2	4
Time	Begins	7:30	7:45	8:00	8:15

272	
129	
83	9.0
7 56 20	0.35 0.74 0.83
46	0.88
72	0.5
8	0.56
93	0.8
143	
58	0.73
S	1 0.63
37	0.8
	0.57
85	0.85
3	0.75
80	0.67 0.85 0.75
2	0.67
7:30-8:30	품

	1 :	т -	1 1 1 1 1 1 1
		104	
78	क	104	96
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46	42	25	49
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17,75.5	2.5 %	. ~	170
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	3/6	李字	22
က	4	4	വ
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8	23	88	9
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12	6	16	12
4	တ	20	16
2	5	က	2.
35	49	52	47
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	建築等		
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	7	7	7
2			_
	22	0	C)
4:30	4:45	5:00	5:15
	4:45	5:00	5:15

4:30-5:30 26 44 56 75 75 76 69 105 105 106 0.77 0.77 0.75 0.89 0.8 0.91	369		
1:30-5:30 26 44 5 75 22 78 5 105 180 12 59 49 120 3 50 16 PHF 0.93 0.65 0.63 0.67 0.63 0.77 0.77 0.75 0.89 0.8	189		
1:30-5:30 26 44 55 75 22 78 55 105 12 59 49 PHF 0.93 0.65 0.63 0.77 0.69 0.67 0.63 0.77 0.6 0.74 0.77	69	0.91	
1:30-5:30 26 44 55 75 22 78 55 105 12 59 49 PHF 0.93 0.65 0.63 0.77 0.69 0.67 0.63 0.77 0.6 0.74 0.77	9	8.0	
1:30-5:30 26 44 55 75 22 78 55 105 12 59 49 PHF 0.93 0.65 0.63 0.77 0.69 0.67 0.63 0.77 0.6 0.74 0.77	S	88.	1
1:30-5:30 26 44 55 75 22 78 55 105 12 59 49 PHF 0.93 0.65 0.63 0.77 0.69 0.67 0.63 0.77 0.6 0.74 0.77		75 0	
1:30-5:30 26 44 55 75 22 78 55 105 12 59 49 PHF 0.93 0.65 0.63 0.77 0.69 0.67 0.63 0.77 0.6 0.74 0.77	04-134 1	ó	١
H:30-5:30 26 44 5 75 22 78 5 105 PHF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0.77	120	0.77	
H:30-5:30 26 44 5 75 22 78 5 105 PHF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0.77	9	11.	
H:30-5:30 26 44 5 75 22 78 5 105 PHF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0.77	6	74 0	
H:30-5:30 26 44 5 75 22 78 5 105 PHF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0.77	7	9	
H:30-5:30 26 44 5 75 22 78 5 105 PHF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0.77		o o	
HF 0.93 0.65 0.63 0.72 0.69 0.67 0.63 0		ŀ	
HF 0.93 0.65 0.63 0.72 0.69 0.67 0.69	180		
1:30-5:30 26 44 5 75 22 78 PHF 0.93 0.65 0.63 0.72 0.69 0.67	-	0.77	
FHF 0.93 0.65 0.63 0.72	105	0	
FHF 0.93 0.65 0.63 0.72	105	0	
1:30-5:30 26 44 5 PHF 0.93 0.65 0.63	105	0.67 0.63 0	
HF 0.93 0.65 0.65	22 78 5 105	0.67 0.63 0	
1:30-5:30 26 44 PHF 0.93 0.65	22 78 5 105	0.69 0.67 0.63 0	
1:30-5:30 PHF	75 22 78 5 105	0.72 0.69 0.67 0.63 0	
1:30-5:30 PHF	75 22 78 5 105	.65 0.63 0.72 0.69 0.67 0.63 0	
	75 22 78 5 105	.65 0.63 0.72 0.69 0.67 0.63 0	

2272 GLEN HAVEN DRIVE DELICH ASSOCIATES LOVELAND, CO 80538 Phone: (970) 669-2061

TABULAR SUMMARY OF VEHICLE COUNTS

Date: 2-16-11

Observer: Joe

Jurisdiction: Fort Collins

Day: Wednesday

Intersection:

Rock Creek/Technology

turn
right
ii C

S = straight L = left turn

Total	Αii	42	37		30
Total	east/west	42	37	41	30
Westbound: Rock Creek	Total	9.	20	26	20
nd:	œ	0	-	0	a
stbou	L S R	30	22	26	20
We					
Eastbound: Rock Creek	Total	12)	(1)	10
:pur	œ				
stbou	တ	0 12	13	12	0 10
Ē	7	0	1	က	0
Total	north/south L	0	0	0	0
Technology	Total	0	0		
nd:	2	0	0	0	0
Southbound:	တ			•	
Sou	7	0	0	0	0
	Total	0	0	0	
:pur	œ				
Northbound	တ				
No	7				
Time	Begins	7:30	7:45	8:00	8:15

150 1 50	
66	0.83
98 0	0.82 0.25
51	0.85
47 0	0.33 0.9
0 0	n/a
0	n/a
0	n/a
0	n/a
0 0 0 0	
7:30-8:30	PHF

3		20	64
42	42	20	46
	98	•	0
0	0	0	0
138	20	61	19
4 5 5 5 5 5 5 5 5 5 5	2		30
24	22	31	30
0	0	0	0
0	5	0	0
0 0	0 5 5	0 0 0	0 0 0
	0	0	0
4:30	4:42	2:00	5:15

4:30-5:3(0 0 0	0	9	2	S	5	201 0	107		- O	92	183	188	
# #		n/a	n/a	0.25	0.25		n/a 0.86	0.86	0.95	35 n/a	a 0.95			
)]			J			J	$\frac{1}{1}$		7		

DELICH ASSOCIATES 2272 GLEN HAVEN DRIVE LOVELAND, CO 80538 Phone: (970) 669-2061

TABULAR SUMMARY OF VEHICLE COUNTS

Date: 11-28-12

Observer: Joe

Day: Wednesday

Jurisdiction: Fort Collins

Intersection:

Lady Moon/Precision

R = right turn S = straight L = left turn

Total	All	8	8	51	32
Total	east/west	9	7	2	9
Westbound: Precision	Total	9	9	•	•
별	2	မ	9	4	9
stbou	ဟ	0	0	0	0
We	_	0	0	-	0
Eastbound: Precision	Total	0 %		0	0
:pur	œ	0	0	0	0
estbo	ဟ	0	0	0	0
ü		0	,	0	0
Total	north/south	42	43	46	26
Lady Moon	Total				6 S
ind:	Я	0	1	-	0
thbou	တ	16	15	19	7
Sou		-	-	٧	-
Northbound: Lady Moon Southbound:	Total	1 2 m	18 Ze	22	80
ınd:	œ	0	0	0	0
rthbou	တ	25	26	24	0 18
No	_	0	0	γ-	0
Time	Begins	7:30	7:45	8:00	8:15

18	
24	
23	96:0
8	0.92
0	n/a
20	.25
-	0.25
o	n/a
0	n/a
	0.25
157	
63	0.75
7	0.5
25	3.75
4	-
94	6.0
•	n/a
93	0.89
	0.25
7:30-8:30	PHF

205	
16	
10	0.63
o	0.56
9	n/a
	0.25
9	0.75
	0.25
e	n/a
io.	0.63
189	
117	0.84
46.0 46.0	0.25
103	0.78
2	.65
72	69.0
9-a-62 kg	
	0.25
	0.68
6	n/a
1:30-5:30	PHF

APPENDIX C

	*	-	*	•	4-	1	1	<u></u>	<u> </u>	\	 	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	†	7"	75	4	7	75	*	7
Volume (veh/h)	72	31	46	2	15	93	94	482	6	43	222	13
Number	7	- 4	14	3	8	18	1	- 6	16	5	2	12
Initial Q (Qb), veh	0	. 0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1,00	1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	190.0	193.7	190.0	193.7	186,3	186.3	186.3	186.3	193.7	186.3	186.3	186.3
Lanes	0	1	0	1	1	1	1	• 1	. 1	. 1	1	1
Cap, veh/h	318	0	179	323	326	277	666	765	677	434	713	606
Arrive On Green	0.15	0.18	0.15	0.18	0.18	0.18	0.10	0.41	0.41	0.07	0.38	0.38
Sat Flow, veh/h	919	-618	1020	1353	1863	1583	1774	_1863	1647	1774	1863	1583
Grp Volume(v), veh/h	121	0	84	2	18	104	111	567	7	51	261	15
Grp Sat Flow(s), veh/h/ln	1502	0 .	1583	1353	1863	1583	1774	1863	1647	1774	1863	1583
Q Serve(g_s), s	2.2	0.0	0.0	0.1	0.3	2.3	1.3	10.1	0.1	0.6	3.9	0.2
Cycle Q Clear(g_c), s	2.8	0.0	0.0	2.9	0.3	2.3	1.3	10.1	0.1	0.6	3.9	0.2
Prop In Lane	0.70		0.64	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	381	0	0	323	326	277	666	765	677	434	713	606
V/C Ratio(X)	0.32	0.00	0.00	0.01	0.06	0.37	0.17	0.74	0.01	0.12	0.37	0.02
Avail Cap(c_a), veh/h	1092	0	0	985	1237	1052	993	2974	2629	674	2832	2407
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	13.6	0.0	0.0	15.8	13.4	14.3	5.4	9.8	6.8	7.2	8.7	7.5
Incr Delay (d2), s/veh	0.5	0.0	0.0	0.0	0.1	8.0	0.1	6.4	0.0	0.1	1.5	0.1
Initial Q Delay(d3),s/veh	0.0 1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/ln Lane Grp Delay (d), s/veh		0.0	0.0	0.0	0.1	8.0	0.3	4.7	0.0	0.2	1.6	0.1
Lane Grp LOS	14.1 B	0.0	0.0	15.8	13.5	15.1	5.5	16.1	6.8	7.3	10.1	7.6
	D	005	 -	<u>B</u> _	<u>B</u>	<u>B</u>	A	B	A	<u> </u>	В	A
Approach Vol, veh/h		205			124	•		685			327	
Approach Delay, s/veh Approach LOS		8.3			14.9			14.3			9.6	
• •		Α			В			В			Α	
Timer								<u> </u>				
Assigned Phs		4			8		1	6		5	2	
Phs Duration (G+Y+Rc), s		11.9			11.9		6.8	21.6		5.7	20.5	
Change Period (Y+Rc), s		6.0			6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s Max Q Clear Time (g_c+l1), s		25.0			25.0		10.0	61.5		7.0	58.5	
Green Ext Time (p_c), s		4.8 1.2			4.9 1.2		3.3 0.1	12.1 3.0	•	2.6 0.0	5.9	4
Intersection Summary					1.2		V. I	5.0		U,U	3.0	
HCM 2010 Ctrl Delay			12.3						<u> </u>	· · · · · · · · · · · · · · · · · · ·		
HCM 2010 LOS			12.3 B									
Notes												

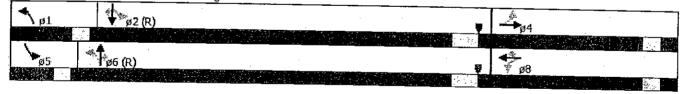
	•		*	1		*	
Phase Number	1	2	4	5	6	8	
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL	
Lead/Lag	Lead	Lag		Lead	Lag	71014	
Lead-Lag Optimize		Ü			Lug		
Recall Mode	None	C-Min	None	None	C-Min	None	
Maximum Split (s)	14	65	31	11	68	31	•
Maximum Split (%)	12.7%	59.1%	28.2%	10.0%	61.8%	28.2%	
Minimum Split (s)	11	32.5	30	11	31.5	31	
Yellow Time (s)	3	4.5	3	3	4.5	3	
All-Red Time (s)	1	2	3	1	2	3	
Minimum Initial (s)	4	7	4	4	7	. 4	•
Vehicle Extension (s)	3	3	3	3	3	3	
Minimum Gap (s)	3	3	3	3	3	3	
Time Before Reduce (s)	. 0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	ő	
Walk Time (s)		7	7		7	7	
Flash Dont Walk (s)		19	17		18	18	
Dual Entry	No	Yes	Yes	No	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	66	80	35	66	77	35	
End Time (s)	80	35	66	77	35	66	
Yield/Force Off (s)	76	28.5	60	73	28,5	60	
Yield/Force Off 170(s)	76	9.5	43	73	10.5	42	
Local Start Time (s)	31	45	0	-31	42	0	
Local Yield (s)	41	103,5	25	38	103.5	25	
Local Yield 170(s)	41	84.5	8	. 38	85,5	7	
Intersection Summary	-						
Cycle Length			110				
Control Type	Actua	ted-Coord					
Matural Cuolo							

Natural Cycle

75

Offset: 35 (32%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red

Splits and Phases: 3: Rock Creek & Ziegler



Lane Configurations		•	-	•	✓	←	•	4	†	<i>*</i>	1	+	4
Lane Configurations Charles (Pethyl) 21 11 21 10 16 82 34 289 8 117 449 Number 7 4 14 3 8 18 1 6 16 6 5 2 Initial Q(bb), weh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Partding Bus Adj 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0		EBL		EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Volume (verhh)	•		₩) N	↑	7	٦	1	7	ሻ	*	7
Initial Q (Qb), veh	• •				10	16	82			_			7 8
Pack-Bike Adj(A_pbT)						8	18	1	6	16	5	2	12
Parking Bus Adj 1.00	• •		0			0	0	0	0	0	0	0	0
Adj Sat Flow veh/h/ln 190.0 193.7 190.0 193.7 190.0 193.7 186.3										1.00	1.00		1.00
Lanes 0 1 0 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1											1.00	1.00	1.00
Cap, veh/h Arrive On Green O.15 O.18 O.18 O.18 O.18 O.18 O.18 O.18 O.18							186.3	186.3	186.3	193.7	186,3	186.3	186.3
Arrive On Green					-			1			1	1	. 1
Sat Flow, veh/h 802 -996 1425 1372 1863 1574 1774 1863 1647 1774 1863 Grp Volume(v), veh/h 38 0 25 12 19 96 40 325 9 138 528 Grp Sat Flow(s), veh/h/n 1559 0 1435 1372 1863 1574 1774 1863 1647 1774 1863 Q Serve(g. s), s 0.0 0.0 0.0 0.3 0.2 0.5 5.1 0.1 1.5 8.9 Cycle Q Clear(g. c), s 0.8 0.0 0.0 1.1 0.3 2.0 0.5 5.1 0.1 1.5 8.9 Prop In Lane 0.66 0.99 1.00 <												751	636
Grp Volume(v), veh/h 38 0 25 12 19 96 40 325 9 138 528 Grp Sat Flow(s), veh/h/in 1559 0 1435 1372 1863 1574 1774 1863 1647 1774 1863 1647 1774 1863 Cycle Q Clear(g_c), s 0.0 0.0 0.0 0.0 0.0 0.3 0.3 0.												0.40	0.40
Grp Sat Flow(s), veh/h/in 1559 0 1435 1372 1863 1574 1774 1863 1647 1774 1863 Q Serve(g, s), s 0.0 0.0 0.0 0.3 0.3 2.0 0.5 5.1 0.1 1.5 8.9 Cycle Q Clear(g, c), s 0.8 0.0 0.0 1.1 0.3 2.0 0.5 5.1 0.1 1.5 8.9 Prop In Lane 0.66 0.99 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 392 0 0 404 328 277 436 667 590 614 751 V/C Ratio(X) 0.10 0.00 0.00 0.03 0.06 0.35 0.09 0.49 0.02 0.22 0.70 Avail Cap(c_a), veh/h 1150 0 0 1109 1285 1086 748 1556 1376 846 1556 14 15 15 15 15 15 15 15 15 15 15 15 15 15									1863	1647	1774	1863	1579
Q Serve(g_s), s										9	138	528	92
Cycle Q Clear(g_c), s										1647	1774	1863	1579
Prop In Lane										0.1	1.5	8.9	1.4
Lane Grp Cap(c), veh/h 392 0 0 404 328 277 436 667 590 614 751 V/C Ratio(X) 0.10 0.00 0.00 0.03 0.06 0.35 0.09 0.49 0.02 0.22 0.70 404 Avail Cap(c_a), veh/h 1150 0 0 1109 1285 1086 748 1556 1376 846 1556 140 HCM Platcon Ratio 1.00			0.0			0.3			5.1		1.5	8.9	1.4
V/C Ratio(X) 0.10 0.00 0.00 0.03 0.06 0.35 0.09 0.49 0.02 0.22 0.70 Avail Cap(c_a), veh/h 1150 0 0 1109 1285 1086 748 1556 1376 846 1556 1 HCM Platoon Ratio 1.00			_										1.00
Avail Cap(c_a), veh/h				-					667			751	636
HCM Platoon Ratio 1.00												0.70	0.14
Upstream Filter(I)												1556	1320
Uniform Delay (d), s/veh 11.8 0.0 0.0 13.6 12.9 13.6 7.2 9.4 7.8 5.4 9.4 Incr Delay (d2), s/veh 0.1 0.0 0.0 0.0 0.0 0.1 0.7 0.1 2.5 0.0 0.2 5.5 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.													1.00
Incr Delay (d2), s/veh												1.00	1.00
Initial Q Delay(d3),s/veh													7.1
%ile Back of Q (50%), veh/ln 0.3 0.0 0.0 0.1 0.1 0.7 0.2 2.2 0.0 0.4 3.9 Lane Grp Delay (d), s/veh 11.9 0.0 0.0 13.6 13.0 14.4 7.3 11.9 7.9 5.6 14.8 Lane Grp LOS B B B B B A B A A B Approach Vol, veh/h 63 127 374 758 Approach Delay, s/veh 7.2 14.1 11.3 12.3 Approach LOS A B B B B B B Timer Assigned Phs 4 8 1 6 5 2 Phs Duration (G+Y+Rc), s 11.6 11.6 5.4 19.0 7.1 20.7 Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+I1), s 2.8 4.0<												5.5	0.5
Lane Grp Delay (d), s/veh 11.9 0.0 0.0 13.6 13.0 14.4 7.3 11.9 7.9 5.6 14.8 Lane Grp LOS B B B B B A B A A A B A A B A A B A A B A A B A A B A A B A A A B A A B A A B A A A B A A B A A A B A B A A A A B A A A B A A A B A A A B A A A B A A A B A A A A B A A A B A A A A B A A A B A A A A B A A A													0.0
Lane Grp LOS B B B B B B A B A A B Approach Vol, veh/h 63 127 374 758 Approach Delay, s/veh 7.2 14.1 11.3 12.3 Approach LOS A B B B B Timer A B 1 6 5 2 Phs Duration (G+Y+Rc), s 11.6 11.6 5.4 19.0 7.1 20.7 Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B 11.9													0.4
Approach Vol, veh/h Approach Delay, s/veh Approach Delay, s/veh Approach LOS A Approach LOS A B B B Timer Assigned Phs As			0.0	0.0									7.6
Approach Delay, s/veh Approach LOS A A Approach LOS A B B B A B B A B Approach LOS B A A B B Approach LOS B A B B Approach LOS B A B B Approach LOS B A B Approach LOS B A B B Approach LOS B A A B B B A B B B A B B B A B		В			B		R	A		A	<u> </u>		A
Approach LOS A B B B Timer Assigned Phs 4 8 1 6 5 2 Phs Duration (G+Y+Rc), s 11.6 11.6 5.4 19.0 7.1 20.7 Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B													
Timer Assigned Phs													
Assigned Phs 4 8 1 6 5 2 Phs Duration (G+Y+Rc), s 11.6 11.6 5.4 19.0 7.1 20.7 Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B			А			R			В			B _.	
Phs Duration (G+Y+Rc), s 11.6 11.6 5.4 19.0 7.1 20.7 Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B	·									··· -	<u></u> -		
Change Period (Y+Rc), s 6.0 6.0 4.0 6.5 4.0 6.5 Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B													
Max Green Setting (Gmax), s 25.0 25.0 8.0 30.5 8.0 30.5 Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay HCM 2010 LOS B													
Max Q Clear Time (g_c+l1), s 2.8 4.0 2.5 7.1 3.5 10.9 Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B													
Green Ext Time (p_c), s 0.7 0.6 0.0 3.3 0.1 3.2 Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B													
Intersection Summary HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B													
HCM 2010 Ctrl Delay 11.9 HCM 2010 LOS B								J. .	5.0		J.1 .	U.L	
HCM 2010 LOS B			-	11.9					 				
											•		
2010/M	Notes			J									

	•	$\tilde{\eta} = \frac{1}{2};$	-		* †	+	
Phase Number	1	2	. 4	5	6	8	
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL	
Lead/Lag	Lead	Lag		Lead	Lag		
Lead-Lag Optimize					_		•
Recall Mode	None	C-Min	None	None	C-Min	None	
Maximum Split (s)	12	37	31	12	37	31	
Maximum Split (%)	15.0%	46.3%	38.8%	15.0%	46.3%	38.8%	
Minimum Split (s)	11	32.5	30	11	31.5	31	
Yellow Time (s)	3	4.5	3	3	4.5	3	
All-Red Time (s)	1	- 2	3	1	2	3	
Minimum Initial (s)	4	7	4	4	7	4	
Vehicle Extension (s)	3	3	3	3	3	3	•
Minimum Gap (s)	3	3	. 3	3	3	3	
Time Before Reduce (s)	. 0	0	0	0	0	0	
Time To Reduce (s)	0	0	. 0	0	0	0	
Walk Time (s)		7	7		7	7.	
Flash Dont Walk (s)		19	17		18	18	
Dual Entry	No	Yes	Yes	No	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	,
Start Time (s)	7	19	56	7	19	56	
End Time (s)	19	56	7	19	56	7	
Yield/Force Off (s)	15	49.5	1	15	49.5	1 -	•
Yield/Force Off 170(s)	15	30.5	64	15	31.5	63	
Local Start Time (s)	31	43	.0	31	43	0	
Local Yield (s)	39	73.5	25	39	73.5	25	•
Local Yield 170(s)	39	54.5	8	39	55.5	7	
Intersection Summary							
Cycle Length			80				

Cycle Length

Control Type

Actuated-Coordinated

Natural Cycle

75

Offset: 56 (70%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red

Splits and Phases: 3: Rock Creek & Ziegler



Intersection	_		_						
Intersection Delay, s/veh	0.2								
Movement	EBL	EBT			WBT	WBR	SBL	SBR	
/ol, veh/h	4	46			91	1	0	0	
Conflicting Peds, #/hr	0	0			0	0	0	0	
Sign Control	Free	Free			Free	Free	Stop	Stop	
RT Channelized	-	None			_	None		None	
Storage Length	-				_	_	0	110110	
Veh in Median Storage, #	-	0			0	_	0		•
Grade, %		0			0	_	. 0	_	
Peak Hour Factor	85	90			85	85	85	85	
Heavy Vehicles, %	2	2			2	2	2	2	
Mvmt Flow	5	5 <u>1</u>			107	1	. 2	. 0	
							J	. 0	
Major/Minor	Major1				Major2		Minor2		
Conflicting Flow All	108	0				0	169	108	·
Stage 1		-			_	_	108	100	
Stage 2	_				_	_	61	,	
Follow-up Headway	2.218	_			_	_	3.518	3.318	
Pot Capacity-1 Maneuver	1483	_			_	_	821	946	
Stage 1	-	_			-	_	916	3 4 0	•
Stage 2	_	_			_	_	962	-	
Time blocked-Platoon, %			٠		_	_	302	-	
Mov Capacity-1 Maneuver	1483	_			_	_	819	946	
Mov Capacity-2 Maneuver	-					_	819	940	•
Stage 1	_	_			_		916		
Stage 2	_					_	959	-	4
- Canada La		•				-	303	-	
Approach	EB				. WB		SB		
HCM Control Delay, s	1				0		0		
Minor Lane / Major Mvmt		EBL	EDT	MDT	WILL	CDI -4			
Capacity (veh/h)		1483	EBT	WBT	WBR	SBLn1			· <u>-</u> -
-departing (verm) -fCM Lane V/C Ratio			-	-		0			
		0.003	-	-	-	+			
ICM Control Delay (s) ICM Lane LOS		7.435	0		-	0		•	
		Α	А			Α			
ICM 95th %tile Q(veh)		0.01	-		-	+			
lotes									

Intersection									
Intersection Delay, s/veh	0.2								
Movement	EBL	EBT			WBT	WBR	SBL	SBR	
Vol, veh/h	0	113			78	1	0	5	
Conflicting Peds, #/hr	0	0			0	0	Õ	0	
Sign Control	Free	Free			Free	Free	Stop	Stop	*
RT Channelized	-	None			-	None	- Ctop	None	
Storage Length	_					-	0	140116	
Veh in Median Storage, #		0			0	_	0	_	
Grade, %		Ō			Ő	-	0		
Peak Hour Factor	85	86			95	85	85	85	e .
Heavy Vehicles, %	2	2			2	2	2	2	
Mvmt Flow	0	131			82	1	0	6	
	•				OL	•		U	
Major/Minor	Мајог1				Major2		Minor2		• • • • • • • •
Conflicting Flow All	. 83	0	-		majorz_	0	214	83	
Stage 1	-	-				_	83	00	
Stage 2	_	· _				_	131		
Follow-up Headway	2.218	_			_	_	3.518	3.318	
Pot Capacity-1 Maneuver	1514				_	_	774	976	
Stage 1	-	_				. <u>-</u>	940	310	-
Stage 2	_	_			•		895		
Time blocked-Platoon, %		-				_	000	_	
Mov Capacity-1 Maneuver	1514	-	•		_		774	976	
Mov Capacity-2 Maneuver		_			_		774	. 010	
Stage 1	· <u>-</u>				_	_	940		
Stage 2	_		·		<u>.</u>	_	895	_	
	•						000		•
Approach	EB		- <u>-</u>	4	WB		SB		
HCM Control Delay, s	0				-0		9		
Minor Lane / Major Mvmt		EBL	EBT	WBT	WBR	SBLn1			
Capacity (veh/h)	****	1514	LDI	AADI	AADL			_	
		IJ 14	_	-	-	976			
HCM Control Dolay (s)		- n	-	-	-	0.006		•	
HCM Control Delay (s) HCM Lane LOS		0	-		-	8.7			•
		A				Α			
HCM 95th %tile Q(veh)		0	-	-	-	0.018			
Notes									
~ : Volume Exceeds Capacit	y; \$: Delay	Exceeds	300 Sec	conds; Er	гог : Com	putation i	Vot Defined		

Intersection							-					
Intersection Delay, s/veh	8.6						-		·			
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	16	18	.12	7	61	21	26	59	3	17	40	5
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	19	21	14	8	72	25	31	69	4	20	47	6
Number of Lanes	0	. 1	1	0	1	0	0	1	ò	1	1	0
								-				
Approach	EB			WB	<u> </u>		NB			SB		
Opposing Approach	WB			EB			SB		<u> </u>	NB		
Opposing Lanes	1			2		-	. 2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			. 1			2			1		
Conflicting Approach Right	NB			SB			WB			EΒ		
Conflicting Lanes Right	1		•	2			. 1			2		
HCM Control Delay	8.1			8.8			9			8,2		
HCM LOS	Α	•		Α	÷		· A			Α		
Lane		NBLn1	EBLn1	EBLn2	WBLn1	SBLn1	SBLn2					
Vol Left, %		30%	47%	0%	8%	100%	0%		 -			
Vol Thru, %		67%	53%	0%	69%	0%	89%					
Vol Right, %		3%	0%	100%	24%	0%	11%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		88	34	12	89	17	45					
LT Vol		59	18	0	61	. 0	40					
Through Vol		3	0	12	21	0	5					
RT Vol		26	16	0	7	17	0					
Lane Flow Rate		104	40	14	105	20	53					
Geometry Grp		6	7	7	6	7	7					
Degree of Util (X)		0.146	0.059	0.017	0.144	0.031	0.073					
Departure Headway (Hd)		5.069	5.327	4.387	4.935	5.537	4.957					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Cap		709	674	817	729	648	724					
Service Time		3.09	3.048	2.109	2.954	3.258	2.678			•		
HCM Lane V/C Ratio		0.147	0.059	0.017	0.144	0.031	0.073					
HCM Control Delay		9	8.4	7.2	8.8	8.4	8.1					
ICM Lane LOS		Α	Α	Α	Α	A	A					
HCM 95th-tile Q		0.5	0.2	0.1	0.5	0.1	0.2					
Votes		•										

^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Intersection												
Intersection Delay, s/veh	8.6											
Intersection LOS	A											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	enn.
Vol, veh/h	11	56	46	3		14	25	40	<u>NDR</u> 5	23		SBR
Peak Hour Factor	0.85	0.85	0.85	0.85	0.89	0.85	0.93	0.85	0.85	23 0.85	.83	5
Heavy Vehicles, %	2	2	2	2	2	2	2	0.00	2	0.00	0.85	0.85
Mymt Flow	13	66	54	4	54	16	27	47	6	27	2 98	2
Number of Lanes	0	. 1	1	ò	1	0	0	1	0	1	96 1	6 0
					•	J	Ů	•	U	1	ı	U
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB	-		SB			NB		
Opposing Lanes	1			2			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			2			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			1			2		
HCM Control Delay	8.2			8,8			9			8.7		
HCM LOS	Α.			Α			Ā			A		
Lane		NBLn1	EBLn1	EBLn2	WBLn1	SBLn1	SBLn2					
Vol Left, %		36%	16%	0%	5%	100%	0%			 -		
Vol Thru, %		57%	84%	0%	74%	0%	94%					
Vol Right, %		7%	0%	100%	22%	0%	6%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					4
Traffic Vol by Lane		70	67	46	65	23	88					
LT Vol		40	56	0	48	0	83					-
Through Vol		5	, 0	46	14	0	5					
RT Vol		25	11	0	3	23	- 0					
Lane Flow Rate		80	79	54	74	27	104			•		
Geometry Grp		6	7	7	6	7	7					
Degree of Util (X)		0.116	0.115	0.067	0.105	0.042	0.146					
Departure Headway (Hd)		5.244	5.242	4.456	5.126	5.631	5.089					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Сар		684	685	804	699	636	705					
Service Time		3.274	2.966	2.18	3.154	3.36	2.817					
HCM Lane V/C Ratio		0.117	0.115	0.067	0.106	0.042	0.148					
HCM Control Delay		9	8.6	7.5	8.8	8.6	8.7					
HCM Lane LOS		Α	Α	Α	Α	Α	A					
HCM 95th-tile Q		0.4	0.4	0.2	0.4	Λ 4	0.5					
HOW SOUTHING Q		0.4	U 1	U.Z	0.4	0.1	0,5					
Notes		0,4	V. 4	0.2	0.4	U. 1 .	0,5					

^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Intersection												
Intersection Delay, s/veh	1.4											
Movement	EBL	EBT	<u>E</u> BR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	1	0	0	1	0	22		95	0	4	61	2
Conflicting Peds, #/hr	0	0	0	0	0		0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop		Free	Free	Free	Free	Free	Free
RT Channelized	•		None	-	_	None	_		None	-	- 100	None
Storage Length	-	-	-	_	-	-	150			150		140110
Veh in Median Storage, #	-	0		_	- 0			0	_	-	0	
Grade, %	-	0	· -	-	0		_	Ö	_	_	. 0	_
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	1	0	26	1	112	0	. 5	72	2
									Ū		12	2
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	209	196	73	196	198	112	74	0	0	112	0	0
Stage 1	82	82	_	114	114			_	-	- 112	U	U
Stage 2	127	. 114		82	84		-	_		_	• -	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	_	-
Pot Capacity-1 Maneuver	748	699	989	763	698	941	1526		_	1478	-	-
Stage 1	926	827	_	891	801		-	_	_	1470	-	
Stage 2	877	801	_	926	825	_	_		_	_	-	-
Time blocked-Platoon, %								-	_	_	_	-
Mov Capacity-1 Maneuver	725	696	989	761	695	941	1526	_	_	1478		-
Mov Capacity-2 Maneuver	725	696	_	761	695		-	_	_	1470	-	-
Stage 1	925	824	_	890	800	_	· <u>-</u>	_		_		-
Stage 2	852	800	_	923	822	_	. <u>-</u>	_		_		
		-					4					-
Approach	€B			WB			NB			SB		
HCM Control Delay, s	10		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	9		·	0	-	<u> </u>	0		
										-		
Minor Lane / Major Mvmt		NBL	NBT	NBR -	EBLn1	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1526	-	-	725	931	1478					
HCM Lane V/C Ratio	٠	0.001	-	_	0.002	0.029	0.003	_	_			
HCM Control Delay (s)		7.361	-	_	10	9	7.444		_			
HCM Lane LOS		Α			В	Ā	A					
HCM 95th %tile Q(veh)		0.002	-		0.005	0.09	0.01		-			
Notes												
	t . D-1		200.0									

^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Intersection												
Intersection Delay, s/veh	1.2							· ·				- -
Movement	EBL	EBT	EBR	WBL	LA II'')	E SAIDIS	NO					
Vol, veh/h	5	0						NBT_	NBR	SBL	SBT	SBR
Conflicting Peds, #/hr	0	0	1	1		9	0	64	1	13	109	1
Sign Control	Stop	Stop		O Ctore		_	_ 0	_ 0	.0	0	0	0
RT Channelized	σιορ	Stop -	Stop None	Stop		•	Free	Free	Free	Free	Free	Free
Storage Length	-		None	-		- None		-	None	-	-	None
Veh in Median Storage, #	-	0	_	_		=	150	-	-	150		-
Grade, %	-	0 0	-	•	(-	. 0	-	-	0	-
Peak Hour Factor	. 85	85	-					0	-	- '	0	_
Heavy Vehicles, %	2		85	85			85	85	85	85	85	85
Mymt Flow	6	2 0	2	2	2		2	2	2	2	2	2
INTERIOR	0	U	1	1	0	11	0	75	1	15	128	1
Major/Minor	Minor2			Minor1			Мајог1			Malawo		
Conflicting Flow All	240	235	129	236	236	76	129	0		Major2	· <u> </u>	
Stage 1	159	159	120	76	76		129		0	76	0	0
Stage 2	81	76	_	160	160		-		•	~		-
Follow-up Headway	3,518	4.018	3.318	3.518	4.018		2.218		-			-
Pot Capacity-1 Maneuver	714	666	921	718	665		1457	-	-	2.218	· -	_
Stage 1	843	766	0 <u>2</u> 1	933	832		140/	-	-	1523	-	-
Stage 2	927	832	•	842	766	· · · · ·	-		-	-	-	-
Time blocked-Platoon, %		352		072	, , , ,		-	-	-	~	.=	-
Mov Capacity-1 Maneuver	701	659	921	712	658	985	1457	-	-	4 500		-
Mov Capacity-2 Maneuver	701	659	-	712	658	200	1407	-	-	1523	-	
Stage 1	843	758	_	933	832	-		-	-	-	-	~
Stage 2	917	832	_	833	758	-	-	-	-		-	
		552		. 000	750		-	-	-	-	-	
Approach	EB			WB			MD		. •			
HCM Control Delay, s	10			9			NB			SB		
Trom Contact Boldy, G	10			. 9			0			1		
Minor Lane / Major Mymt		NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1457			730	949	1523	ODI	UDIX			
HCM Lane V/C Ratio		-	_	-	0.01	0.012	0.01	-	-			
HCM Control Delay (s)		0	-	-	10	8.8	7.388	-	-			
HCM Lane LOS		À			В	Α	7.500 A	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.029	0.038	0.03	-				
Notes		•					2,50		-			
au : Volumo Evacado Canació	- A - D - I											

UNSIGNALIZED INTERSECTIONS

Level-of-Service	Average Total Delay sec/veh
ΑΑ	< 10
B	> 10 and < 15
C	> 15 and < 25
D	> 25 and < 35
E	> 35 and ≤ 50
F	> 50

SIGNALIZED INTERSECTIONS

Level-of-Service	Average Total Delay sec/veh
Α	< 10
В	> 10 and < 20
C	> 20 and < 35
D	> 35 and < 55
E	> 55 and < 80
F	> 80

Table 4-3 Fort Collins (City Limits) Motor Vehicle LOS Standards (Intersections)

	Land	Use (from st	ructure plan)						
		Other corridors within:							
Intersection type	Commercial corridors	Mixed use districts	Low density mixed use residential	All other areas					
Signalized intersections (overall)	D	E*	D	D					
Any Leg	E	E	D	E					
Any Movement	E	E	D	E					
Stop sign control (arterial/collector or local— any approach leg)	N/A	F**	F**	E					
Stop sign control (collector/local—any approach leg)	N/A	C	С	C					
 mitigating measures required considered normal in an urban 	n environment			7					

APPENDIX D

	→ ,	-	•	€	←	•	1	†	/	/	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL.	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₩		'n	†	Ţ.	Ť	1	7.	ሻ	↑	7
Volume (veh/h)	78	43	50	17	20	102	101	519	39	76	239	84
Number	. 7	4	14	3	8	18	1	6	- 16	5	2	12
Initial Q (Qb), veh	0	0	. 0	0	0	0	0	0	-0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00	_	1.00
Parking Bus Adj	1.00	1.00	1,00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	190.0	193.7	190.0	193.7	186.3	186.3	186.3	186.3	193.7	186.3	186.3	186,3
Lanes	0	1	0	1	1	1	1	1	1	1	1	1
Cap, veh/h	290	0	166	306	359	305	635	799	706	419	782	665
Arrive On Green	0.17	0.19	0.17	0.19	0.19	0.19	0.09	0.43	0.43	0.08	0.42	0.42
Sat Flow, veh/h	826	-315	864	1329	1863	1583	1774	1863	1647	1774	1863	1583
Grp Volume(v), veh/h	142	. 0	110	20	24	115	119	611	46	89	281	99
Grp Sat Flow(s),veh/h/ln	1527	0	1610	1329	1863	1583	1774	1863	1647	1774	1863	1583
Q Serve(g_s), s	2.7	0.0	0.0	0.6	0.5	2.9	1.6	12.7	0.7	1.2	4.7	1.8
Cycle Q Clear(g_c), s	3.7	0.0	0.0	4.3	0.5	2.9	1.6	12.7	0.7	1.2	4.7	1.8
Prop In Lane	0.65		0.54	1.00	0.0	1.00	1.00	12.1	1.00	1.00	4.7	1.00
Lane Grp Cap(c), veh/h	391	0	0	306	359	305	635	799	706	419	782	665
V/C Ratio(X)	0.36	0.00	0.00	0.07	0.07	0.38	0.19	0.76	0.07	0.21	0.36	0.15
Avail Cap(c_a), veh/h	947	0	0	810	1065	905	904	2561	2264	587	2438	2072
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.1	0.0	0.0	18.3	15.0	16.0	6.0	11.0	7.6	7.8	9.0	
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.1	0.1	0.8	0.1	6.9	0.2	0.3	1.3	8.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5
%ile Back of Q (50%), veh/ln	1.3	0.0	0.0	0.2	0.2	1.1	0.5	5.8	0.0	0.0	1.9	0.0
Lane Grp Delay (d), s/veh	15.7	. 0.0	0.0	18.3	15.1	16.7	6.1	17.9	7.8	8.1		0.6
Lane Grp LOS	В		5,5	В	В	В	Α	17.3 B	7.0 A	0. I A	10.3 B	8.6
Approach Vol, veh/h		252	-		159		- 71	776		A		· A
Approach Delay, s/veh		8.8			16.7			15.5			469	
Approach LOS		A			В			13,3 B			9.5	
Timer		• •			, ,			D			Α	
Assigned Phs		4			8		1	6		 5	2	
Phs Duration (G+Y+Rc), s		13.8			13.8		7.1	25.0		6.7		
Change Period (Y+Rc), s		6.0			6.0		4.0	6.5		4.0	24.6	
Max Green Setting (Gmax), s		25.0			25.0		10.0	61.5			6.5	
Max Q Clear Time (g_c+l1), s		5.7			6.3		3.6	14.7		7.0 3.2	58.5	
Green Ext Time (p_c), s		1.5			1.5		0.1	3.8		3.2 0.1	6.7 3.8	
Intersection Summary								• •		3.1		
HCM 2010 Ctrl Delay			12.9							<u> </u>		
HCM 2010 LOS			В									
Notes			_									
NUCS												

	1	↓ e	-	\	e †	4 -	
Phase Number	1	2	4	5	6	8	
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL	
Lead/Lag	Lead	Lag		Lead	Lag		
Lead-Lag Optimize					Ū		
Recall Mode	None	C-Min	None	None	C-Min	None	•
Maximum Split (s)	14	65	31	11	68	31	•
Maximum Split (%)	12.7%	59.1%	28.2%	10.0%	61.8%	28.2%	
Minimum Split (s)	11	32.5	30	11	31.5	31	
Yellow Time (s)	3	4.5	3	3	4.5	3	
All-Red Time (s)	1	2	3	1	2	3	
Minimum Initial (s)	4	7	4	4	7	4	•
Vehicle Extension (s)	3	3	3	3	3	3	
Minimum Gap (s)	3	3	3	3	3	3	
Time Before Reduce (s)	. 0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	. 0	
Walk Time (s)		7	7		7	• 7	
Flash Dont Walk (s)		19	17		18	18	
Dual Entry	No	Yes	Yes	No	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes.	Yes	
Start Time (s)	66	- 80	35	66	77	35	
End Time (s)	80	35	66	77	35	66	
Yield/Force Off (s)	76	28.5	60	73	28.5	60	
Yield/Force Off 170(s)	76	9,5	43	73	10.5	42	
Local Start Time (s)	31	45	0	31	42	0	
Local Yield (s)	41	103.5	25	38	103.5	25	•
Local Yield 170(s)	41	84.5	8	38	85.5	7	
Intersection Summary							
O. 4-1- 14h-							

Cycle Length

110

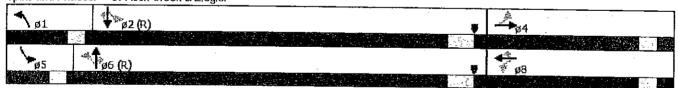
Control Type

Actuated-Coordinated

Natural Cycle

Offset: 35 (32%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red

3: Rock Creek & Ziegler Splits and Phases:



	۶	→	•	•	◄	•	•	†	1	-	Į.	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		<u> </u>	<u></u>	- ₹	75	<u>↑</u>	7	ሻ	†	7
Volume (veh/h)	23	17	23	43	26	103	37	311	24	131	483	84
Number	7	4	14	3	8	18	1	. 6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	.0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.96	0.97		0.99	1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	190.0	193.7	190.0	193.7	186.3	186.3	186.3	186.3	193.7	186.3	186.3	186.3
Lanes	. 0	1	0	1	1	1	1	1	1	1	1	1
Cap, veh/h	272	0	230	410	367	310	406	696	615	588	773	656
Arrive On Green	0.17	0.20	0.17	0.20	0.20	0.20	0.06	0.37	0.37	0.10	0.42	0.42
Sat Flow, veh/h	688	-535	1167	1366	1863	1575	1774	1863	1647	1774	. 1863	1580
Grp Volume(v), veh/h	47	0	35	51 .	31	121	44	349	28	154	568	99
Grp Sat Flow(s),veh/h/ln	1579	0	1503	1366	1863	1575	1774	1863	1647	1774	1863	1580
Q Serve(g_s), s	0.0	0.0	0.0	1.3	0.6	2.8	0.6	6.0	0.4	1.8	10.6	1.6
Cycle Q Clear(g_c), s	1.0	0.0	0.0	2.3	0.6	2.8	0.6	6.0	0.4	1.8	10.6	1.6
Prop In Lane	0.57		0.78	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	409	0	0	410	367	310	406	696	615	588	773	656
V/C Ratio(X)	0.11	0.00	0.00	0.12	80.0	0.39	0.11	0.50	0.05	0.26	0.73	0.15
Avail Cap(c_a), veh/h	1051	0	0	998	1168	988	680	1415	1251	788	1415	1200
HCM Platoon Ratio	1.00	1.00	1.00	1,00	1,00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	12.3	0.0	0.0	14.7	13.6	14.5	7.8	10.0	8.3	5,9	10.2	7.6
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.1	0.1	8.0	0.1	2.6	0.1	0.2	6.1	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/ln	0.3	0.0	0.0	0.4	0.2	1.0	0.2	2.4	0.2	0.5	4.9	0.6
Lane Grp Delay (d), s/veh	12.4	0.0	0.0	14.9	13.7	15.3	7.9	12.6	8.4	6.1	16.3	8.1
Lane Grp LOS	В			В	В	В	Α	- B	Α	Α	В	Α
Approach Vol, veh/h		82		-	203			421			821	
Approach Delay, s/veh		7.1			14.9			11.8			13.4	
Approach LOS		Α			В			В			В	
Timer											_	
Assigned Phs		4			8		1	6		5	2	
Phs Duration (G+Y+Rc), s		13.2			13.2		5.6	21.0		7.3	22.7	
Change Period (Y+Rc), s		6.0			6.0		4.0	6.5		4.0	6.5	
Max Green Setting (Gmax), s		25.0			25.0		8.0	30.5		8.0	30.5	
Max Q Clear Time (g_c+l1), s		3.0		•	4.8		2.6	8.0		3.8	12.6	
Green Ext Time (p_c), s		1.0			1.0		0.0	3.6		0.1	3.5	
Intersection Summary							2.0	3.4		5.1	0.0	
HCM 2010 Ctrl Delay			12.8			- · · · · · · · · · · · · · · · · · · ·	 .		·		_ _	
HCM 2010 LOS			12.0 B									
			ղ									
Notes												

	•	# \$	-	-	⇔ †.	+	
Phase Number	1	2	4	5	6	. 8	
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL	
Lead/Lag	Lead	Lag		Lead	Lag		
Lead-Lag Optimize					3		
Recall Mode	None	C-Min	None	None	C-Min	None	
Maximum Split (s)	12	37	31	12	37	31	
Maximum Split (%)	15.0%	46.3%	38.8%	15.0%	46.3%	38.8%	
Minimum Split (s)	11	32.5	30	11	31.5	31	•
Yellow Time (s)	3	4.5	3	3	4.5	3	
All-Red Time (s)	1	2	3	1	2	. 3	•
Minimum Initial (s)	4	7	4	4	7	4	
Vehicle Extension (s)	3	3	3	3	3	3	
Minimum Gap (s)	3	3	3	3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	0	
Walk Time (s)		7	. 7		7	7	
Flash Dont Walk (s)		19	17		18	18	
Dual Entry	No	Yes	Yes	No	Yes	Yes	•
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	7	19	-56	. 7	19	56	
End Time (s)	19	56	7	19	56	7	
Yield/Force Off (s)	15	49.5	1	15	49,5	1	•
Yield/Force Off 170(s)	1 5	30.5	64	. 15	31.5	63	
Local Start Time (s)	31	43	Ò	31	43	0	
Local Yield (s)	39	73.5	25	39	73.5	25	•
Local Yield 170(s)	39	54.5	8	39	55.5	7	
Intersection Summary	· .			•			
Cycle Length	-		80				
Control Type	Actua	ted-Coor	dinated				

Control Type

Actuated-Coordinated

Natural Cycle

75

Offset: 56 (70%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red

Splits and Phases: 3: Rock Creek & Ziegler



Intersection									
ntersection Delay, s/veh	1.4						· · ·		
Movement	EBL	EBT			WBT	WBR	SBL	SBR	
/ol, veh/h	45	89			144	9	3	4	
Conflicting Peds, #/hr	0	0			0	0	0	0	•
sign Control	Free	Free			Free	Free	Stop	Stop	
RT Channelized	-	None			-	None	-	None	
Storage Length	_	-				_	0	-	
/eh in Median Storage,#	-	0			0	-	0	_	
Grade, %	-	0		•	0	_	0	_	•
Peak Hour Factor	85	90			85	85	85	85	
leavy Vehicles, %	2	2			2	. 2	. 2	2	
/wmt Flow	53	99			169	11	4	5	
//ajor/Minor	Major1		<u> </u>	·	Major2	·	Minor2		
Conflicting Flow All	180	0			-	0	380	175	
Stage 1	-	-			-	_	175	=	
Stage 2		-			-		205	_	
ollow-up Headway	2.218	-			-	_	3.518	3.318	
ot Capacity-1 Maneuver	1396	-				_	622	868	
Stage 1	· -	-			-	_	855	-	
Stage 2	-	-			_	_	829		٠
ime blocked-Platoon, %		· -				_			
Nov Capacity-1 Maneuver	1396				_	_	597	868	
Nov Capacity-2 Maneuver	. 2				_		597	-	
Stage 1					-	-	855	_	•
Stage 2	_	· <u>-</u>		•		_	796	_	
						٠	700	-	
pproach	EB				WB		SB		
CM Control Delay, s	3				0		10	·	
liner Lene / Maian Maray		EDI	CP.	1455	1.0 (**		•	•	
linor Lane / Major Mvmt		EBL	EBT	WBT	WBR	SBLn1			
apacity (veh/h)		1396	-	-	-	727			
CM Lane V/C Ratio		0.038	-	-	-	0.011			
CM Control Delay (s)		7.68	0	-	-	10			
CM Lane LOS		A	Α.			В			
CM 95th %tile Q(veh)		0.118	-	-	-	0.034			
otes									
: Volume Exceeds Capacit	v \$ · Delay	/ Evcoods	300 900	ondo: Fer		nutotion A	D - C	· · · · · · · · · · · · · · · · · · ·	

Intersection										
ntersection Delay, s/veh	1.4									
Movement	EBL	EBT			WBT	WBR	SBL	SBR		
Vol, veh/h	7	175			145	2	17	29		
Conflicting Peds, #/hr	0	0			0	0	0	0		
Sign Control	Free	Free			Free	Free	Stop	Stop		
RT Channelized	-	None			-	None		None		
Storage Length	-	-			_	· -	0	•		
Veh in Median Storage,#	-	. 0			0	_	0			
Grade, %	-	0			0	_	0			
Peak Hour Factor	85	86			95	85	85	85		
Heavy Vehicles, %	2	2			2	. 2	2	2		
Avmt Flow	. 8	203			153	2	20	34		
Vajor/Minor	Major1			•	Major2		Minor2			
Conflicting Flow All	155	0				- 0	374	154		
Stage 1	-	-			_		154	134		
Stage 2	_	 .				_	220	-		
ollow-up Headway	2.218				_		3.518	3.318		
ot Capacity-1 Maneuver	1425	_				_	627	892		
Stage 1	-	_				_	874	032		
Stage 2	-	_					817	_		
ime blocked-Platoon, %		_								
Nov Capacity-1 Maneuver	1425				_	_	623	892		•
lov Capacity-2 Maneuver	-				_	_	623	-		•
Stage 1	_	_				_	874	_		
Stage 2	-	_			_	-	812	_		
							-O12	_		
pproach	EB		-	•	WB		SB			
ICM Control Delay, s	0				. 0	•	10			
linor Lane / Major Mvmt		EBL	EBT	WBT	WBR	SBLn1				
apacity (veh/h)		1425			_	769				
ICM Lane V/C Ratio		0.006	_	_	-	0.07				
CM Control Delay (s)		7.541	0	-	-	10				
CM Lane LOS	•	Α	Α			В		·	•	•
CM 95th %tile Q(veh)		0.017	-	-	-	0.227				
otes	· 								•	
: Volume Exceeds Capacit	v: \$ · Delay	Exceeds	300 Sec	onde: En	ror · Com	nufation N	lat Defined			 _

Intersection	•											
Intersection Delay, s/veh	9.7							·			· .	
Intersection LOS	Α							•				
Movement	. EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	e D D
Vol, veh/h	52	24	16	8	83	50	36	93	3	27	58	SBR 34
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	0.03	0.65	0.85
Mvmt Flow	61	28	19	9	98	59	42	109	4	32	68	2
Number of Lanes	0	1	1	0	1	0	0	1	0	1	1	40 0
Approach	EB			WB			NB					
Opposing Approach	WB			EB	· · · · · ·		SB			SB		
Opposing Lanes	1			2			SB 2			NB		
Conflicting Approach Left	SB	-		NB			EB			1		
Conflicting Lanes Left	2			1			2			WB		
Conflicting Approach Right	NB			SB		•	WB			T		
Conflicting Lanes Right	1			2			• 1			EB		
HCM Control Delay	9,3			10			10.3	•		2		
HCM LOS	A			A			10.3 B			8,9 A		
Lane		NBLn1	EBL _n 1	EDI 50	WBLn1	CDI =4	:0.01 0					
Vol Left, %	 -	27%	68%	0%		SBLn1	SBLn2					
Vol Thru, %		70%	32%	0% 0%	6%	100%	0%					
/ol Right, %		2%	0%	100%	59%	0%	63%					
Sign Control		Stop	Stop		35%	0%	37%					
Fraffic Vol by Lane		132	3.0p 76	Stop 16	Stop	Stop	Stop					
_T Vol		93	24	0	141	27	92					
Through Vol		-33	- 0	16	83 50	0	58					
RT Vol		36	52	0	. 8	0 27	34					
ane Flow Rate		155	89	19	166	32	100					
Geometry Grp		6	7	7	6	32 7	108 7		-			
Degree of Util (X)		0.237	0.145	0.025	0.243	0.052	0.156					
Departure Headway (Hd)		5.495	5.845	4.794	5.269	5.946	5.181					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes				•	
Cap		649	610	740	677	599	687		-			
Service Time		3.561	3.615	2.564	3.334	3.712	2.947					
ICM Lane V/C Ratio		0.239	0.146	0.026	0.245	0.053	2.94 <i>1</i> 0.157					
ICM Control Delay		10.3	9.6	7.7	10	0.000	0.157 8.9					•
ICM Lane LOS		В	3.0 A	Α.,	A	A	6.9 A					
ICM 95th-tile Q		0,9	0.5	0.1	0.9	0.2	0.6					
lotes												

^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Intersection												
Intersection Delay, s/veh	10.1								· ·			
Intersection LOS	В											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	41	86	65	3	68	30	35	61	6	49	125	44
Peak Hour Factor	0.85	0.85	0.85	0.85	0.89	0.85	0.93	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2.	2	2	2	2	2	2	2	2	2
Mvmt Flow	48	101	76	4	76	35	38	72	7	58	147	52
Number of Lanes	0	1	1	0	1	0	0	1	0	1	1	0
Approach	EB		ř	WB			NB			SB	• •	•
Opposing Approach	WB			EB			SB			NB	· · · ·	
Opposing Lanes	1			2			2			1		
Conflicting Approach Left	SB			NB			EB	:		WB		
Conflicting Lanes Left	2			1			2			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			1			2		
HCM Control Delay	9.8			10.1			10.3			10.3		
HCM LOS	Α			В		÷	В		•	В		
Lane		NBLn1	EBLn1	EBLn2	WBLn1	SBLn1	SBLn2	-				
Vol Left, %		34%	32%	0%	3%	100%	0%					
Vol Thru, %		60%	68%	0%	67%	0%	74%					
Vol Right, %		6%	0%	100%	30%	0%	26%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		102	127	65	101	49	169					
_T Vol		61	86	0	68	0	125	-				
Through Vol		6	0	65	30	0	44		*,			
RT Vol		35	41	0	3	49	0					
.ane Flow Rate		116	149	76	115	58	199					
Geometry Grp		6	7	7	6	7	7	•	•			
Degree of Util (X)		0.192	0.242	0.105	0.185	0.097	0.298		*			
Departure Headway (Hd)		5.922	5.933	5.063	5.769	6.179	5.49					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Сар		609	610	712	625	583	658					
Service Time		3.93	3.633	2.763	3.777	3.879	3.19					
ICM Lane V/C Ratio		0.19	0.244	0.107	0:184	0.099	0.302					
HCM Control Delay		10.3	10.5	8.4	10.1	9.5	10.5	:				
ICM Lane LOS		В	В	A	В	. A	В					
HCM 95th-tile Q		0.7	0.9	0.4	0.7	0.3	1.2	٠				
Votes												

Intersection	·											
Intersection Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	. WBT	WBR	NBL	NDT	NOD	0.01	257	
Vol, veh/h	4	0	<u>:::::10</u>	13				NBT 404	NBR	SBL	SBT	SBF
Conflicting Peds, #/hr	Ö	0	0	0			0	191	2	11	106	18
Sign Control	Stop	Stop	Stop	Stop				0	0	_ 0	- 0	_ (
RT Channelized	- CiOp	- -	None	utop	SiOp.		Free	Free	Free	Free	Free	Free
Storage Length	_	_	140110	_	•	. MONE	- 150	•	None	- 4¢0	-	None
Veh in Median Storage, #	_	0	_	-	0		150	-	-	150	-	-
Grade, %		0	_	_	_		-	0	-	-	0	•
Peak Hour Factor	85	85	85	85			85	0 85	۰-	~	0	
Heavy Vehicles, %	2	2	2	2			2		85	85	85	85
Mvmt Flow	5	0	0	15	1		2	2 225	2 2	2 13	2 125	2 21
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	420	393	135	392	403	226	146	0	0	227		0
Stage 1	161	161	-	231	231	-	_	-			-	-
Stage 2	259	232	-	161	172	. -	-	~	-	_	_	
Follow-up Headway	3.518	4.018	3.318	3,518	4.018	3.318	2.218	-	-	2.218	_	_
Pot Capacity-1 Maneuver	544	543	914	567	536	813	1436	-	_	1341	_	_
Stage 1	841	765	-	772	713	-	_		-	-	_	_
Stage 2	746	713	-	841	756	-	-	_	_	_	-	_
Time blocked-Platoon, %								-			-	
Mov Capacity-1 Maneuver	502	537	914	562	530	813	1436	-	_	1341	_	_
Mov Capacity-2 Maneuver	502	537	-	562	530	-	-		_		_	-
Stage 1	840	758	-	771	712	-	-	_	~	_	, _	_
Stage 2	693	712		833	749	-	-	-		-	* <u>*</u>	-
Approach	EB			. WB			NB			SB		
HCM Control Delay, s	12			10			0	, ·		1	<u></u>	
Minor Lano / Major Muset	٠	NDI	NDT	NDD	ED: 1							
Minor Lane / Major Mymt	·	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		· <u></u>	
Capacity (veh/h)		1436	-		502	736	1341	-	-			
HCM Control Delou (a)		0.002	-	-	0.009	0.098	0.01	-	-			
HCM Control Delay (s) HCM Lane LOS		7.511	-	-	12.2	10.4	7.711	-	-			
		Α			В	В	Α					
HCM 95th %tile Q(veh)		0.005	-	-	0.028	0.323	0.029	-	-			
Votes							·					
: Volume Exceeds Capacit	y; \$: Delay	/ Exceeds	300 Sec	conds; Er	тог : Con	putation 1	Vot Define	ed				
	,,,,				. 51 . 5511	Paralloi I	ANT DOUBLE	u				

Intersection Intersection Delay, s/veh	2,1											
mersection belay, siven	2.1				٠							
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Vol, veh/h	24	. 1	2	6			0	119	13	39	210	ODI
Conflicting Peds, #/hr	0	0	0	0	C		.0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop			Free	Free	Free	Free	Free	Free
RT Channelized	-		None	, -		None	-	- 1100	None	1166	1166	None
Storage Length	-	_	_	_	_	-	150	_	-	150		NOTE
Veh in Median Storage, #	-	0	_	·	0	_	100	0	_	130	- n	
Grade, %	_	0	· · <u>-</u>	-	Ö			0	-		0	
Peak Hour Factor	85	85	85	85	85		85	85	85	85°	0	0.0
Heavy Vehicles, %	2	2	2	2	2		2	2	2		85	85
Mvmt Flow	28	1	2	7	0	26	0	140	15	2 46	2 247	į
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	502	496	249	491	492	140	Major1			Major2		
Stage 1	341	341	240	148	148	148	252	0	0	155	0	(
Stage 2	161	155	-	343	344		-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	2.240	0.040	-	- ,			-
Pot Capacity-1 Maneuver	480	475	790	488		3.318 899	2.218		~	2.218	~	
Stage 1	674	639	130	855	478 775	699	1313	-	-	1425	· -	
Stage 2	841	769	-	672	637	-		-	-		-	-
Time blocked-Platoon, %	041	103		072	637	-	-	-	-	-		-
Mov Capacity-1 Maneuver	455	460	790	474	400	000	1040	-	-		-	-
Mov Capacity-2 Maneuver	455	460	130	474	463 463	899	1313	-	-	1425	-	-
Stage 1	674	618	-	855		-	-	-		-	~	-
Stage 2	817	769	-	647	775 616	-	-	-	-			-
Clago Z	017	109	-	041	010	· -	-	-	-		-	-
Approach	EB			WB		_	NB			SB		
HCM Control Delay, s	13			10			0			1		
Minor Lane / Major Mymt		NBL.	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1313			470	754	1425	ODI		· — -		
HCM Lane V/C Ratio			-	-	0.068	0.044	0.032	-	-			
HCM Control Delay (s)		0	-		13.2	10	7.61	-	-			
-ICM Lane LOS		·A		-	13.2 B	В		-	,-			
HCM 95th %tile Q(veh)		0	=		0.217	0.137	A 0.1		_			
lotes		-			,•		9,1		-			
: Volume Exceeds Capacity	r C Dolov	Evacada	200.0	1 . =		 .						

Delich Associates

APPENDIX E

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Movement	EBL_	EBT	EBR	WBL	WBT	WBR	NBĹ	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		1	↑	7	7		7	ነኝ		7
Volume (veh/h)	78	44	50	18	20	104	101	519	42	84	239	84
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	. 0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00	Ū	1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	190.0	193.7	190.0	193.7	186.3	186.3	186.3	186.3	193.7	186.3	186.3	186.3
Lanes	0	- 1	0	1 -	1	1	1	1	1	1	. 1	1
Cap, veh/h	288	0	166	304	360	306	636	798	705	422	787	669
Arrive On Green	0.17	0.19	0.17	0.19	0.19	0.19	0.09	0.43	0.43	0.08	0.42	0.42
Sat Flow, veh/h	821	302_	857	1328	1863	1583	1774	1863	1647	1774	1863	1583
Grp Volume(v), veh/h	142	0	111	21	24	117	119	611	49	99	281	99
Grp Sat Flow(s),veh/h/ln	1527	0	1612	1328	1863	1583	1774	1863	1647	1774	1863	1583
Q Serve(g_s), s	2.7	0.0	0.0	0.7	0.5	3.0	1.6	12.8	0.8	1.3	4.7	1.8
Cycle Q Clear(g_c), s	3.8	0.0	0.0	4.4	0.5	3.0	1.6	12.8	0.8	1.3	4.7	1.8
Prop In Lane	0.65		0.53	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	391	0	0	304	360	306	636	798	705	422	787	669
V/C Ratio(X)	0.36	0.00	0.00	0.07	0.07	0.38	0.19	0.77	0.07	0.23	0.36	0.15
Avail Cap(c_a), veh/h	938	0	. 0	800	1056	897	902	2537	2243	581	2416	2053
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.2	0.0	0.0	18.4	15.1	16.1	6.0	11.2	7.7	7.9	9.0	8.2
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.1	0.1	8.0	0.1	6.9	0.2	0.3	1.3	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q (50%), veh/ln	1.3	0.0	0.0	0.2	0.2	0.1	0.5	5.9	0.3	0.4	1.9	0.6
Lane Grp Delay (d), s/veh	15.8	0.0	0.0	18.5	15.2	16.9	6.2	18.1	7.9	8.2	10.3	8.6
Lane Grp LOS	<u>B</u>			В	В -	В	A	В	Α	A.	В	А
Approach Vol, veh/h		253			162			779	·		479	
Approach Delay, s/veh		8.9			16.9			15.6			9.5	
Approach LOS		Α			В			В			Α	
Timer												
Assigned Phs		. 4			8		1	6		5	2	
Phs Duration (G+Y+Rc), s		13.9			13.9		7.1	25.2		6.9		
Change Period (Y+Rc), s		6.0			6.0		4.0	6.5		4.0	24.9 6.5	
Max Green Setting (Gmax), s		25.0			25.0		10.0	61.5		7.0	58.5	
Max Q Clear Time (g_c+l1), s		5.8			6.4		3.6	14.8		3.3	6.7	
Green Ext Time (p_c), s		1.5			1.5		0.1	3.8		0.1	3.9	
ntersection Summary												
HCM 2010 Ctrl Delay			13.0									
HCM 2010 LOS			В									
Notes												

	•	9	A	<u> </u>	*	*				·		 	
Phase Number	1	2	4	5	6	8							
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL						 	
Lead/Lag	Lead	Lag		Lead	Lag		-						
Lead-Lag Optimize													
Recall Mode	None	C-Min	None	None	C-Min	None							
Maximum Split (s)	14	65	31	11	68	31							
Maximum Split (%)	12.7%	59.1%	28.2%	10.0%	61.8%	28.2%							
Minimum Split (s)	11	32.5	30	11	31.5	31							
Yellow Time (s)	3	4.5	. 3	3	4.5	. 3							
All-Red Time (s)	1	2	3	1	2	3							
Minimum Initial (s)	4	. 7	4	4	7	4					,		
Vehicle Extension (s)	3	3	3	3	3	3							
Minimum Gap (s)	3	3	3	3	3	3							
Time Before Reduce (s)	. 0	0	. 0	0	0	0							
Time To Reduce (s)	0	0	0	0	. 0	0						•	
Walk Time (s)		7	7		7	7							
Flash Dont Walk (s)		19	17		18	18					•		
Dual Entry	No	Yes	Yes	No	Yes	Yes							
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes							
Start Time (s)	66	80	35	66	. 77	35							
End Time (s)	80	35	66	77	35	66							
Yield/Force Off (s)	76	28.5	60	73	28.5	60			•				
Yield/Force Off 170(s)	76	9.5	43	73	10.5	42							
Local Start Time (s)	31	45	0	31	42	0							
Local Yield (s)	41	103.5	25	38	103.5	25							
Local Yield 170(s)	41	84.5	8	38	85.5	7		٠					
Intersection Summary													
Cycle Length		<u> </u>	110									 	
Control Type	A =4	1 10	1										

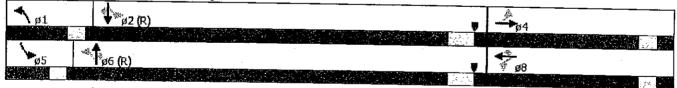
Control Type

Actuated-Coordinated

Natural Cycle

Offset: 35 (32%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red

Splits and Phases: 3: Rock Creek & Ziegler



	→	-	-	•	←	•	1	†	<u> </u>	-	1	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		*5	↑	7	ሻ	†	7	<u> </u>		7
Volume (veh/h)	23	17	23	46	27	111	37	311	25	133	483	84
Number	7	4	14	.3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.96	0.97		1.00	1.00		1.00	1.00	Ū	1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow veh/h/ln	190.0	193.7	190.0	193.7	186.3	186.3	186.3	186.3	193.7	186.3	186.3	186.3
Lanes	0	1	0	1	1	1	1	1	1	100.0	100.5	
Cap, veh/h	275	0	234	417	377	319	402	692	612	583	771	654
Arrive On Green	0.18	0.20	0.18	0.20	0.20	0.20	0.06	0.37	0.37	0.10	0.41	0.41
Sat Flow, veh/h	690	-533	1156	1367	1863	1576	1774	1863	1647	1774	1863	
Grp Volume(v), veh/h	47	0	35	54	32	131	44	349	29			1580
Grp Sat Flow(s),veh/h/ln	1568	0	1508	1367	1863	1576	1774	1863	. 1647	156	568	99
Q Serve(g_s), s	0.0	0.0	0.0	1.4	0.6	3.0	0.6	6.1	0.5	1774	1863	1580
Cycle Q Clear(g_c), s	1.0	0.0	0.0	2.4	0.6	3.0	0.6	6.1		1.9	10.8	1.6
Prop In Lane	0.57		0.77	1.00	0.0	1.00	1.00	0.1	0.5	1.9	10.8	1.6
Lane Grp Cap(c), veh/h	415	0	0	417	377 .	319	402	692	1.00	1.00	~	1.00
V/C Ratio(X)	0.11	0.00	0.00	0.13	0.08	0.41	0.11		612	583	771	654
Avail Cap(c_a), veh/h	1033	0.00	- 0	987	1154	976	672	0.50	0.05	0.27	0.74	0.15
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1398	1236	779	1398	1186
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	12.2	0.0	0.0	14.7	13.6	14.6	1.00	1.00	1.00	1.00	1.00	1.00
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.1	0.1	0.8	7.9	10.2	8.4	6.0	10.4	7.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0		0.1	2.6	0.1	0.2	6.2	0.5
%ile Back of Q (50%), veh/ln	0.3	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane Grp Delay (d), s/veh	12.3	0.0	0.0	14.9		1.1	0.2	2.6	0.2	0.5	5.0	0.6
Lane Grp LOS	12.3 B	0.0	U.U		13.7	15.4	8.0	12.8	8.6	6.2	16.6	8.2
Approach Vol, veh/h				В	B	B	A	В	A	<u>`</u> A	<u>B</u>	<u>A</u>
Approach Delay, s/veh		82			217			422			823	
		7.0			15.0			12.0			13.6	
Approach LOS		Α			В			В			B.	
Timer										_	•	
Assigned Phs		4			8		1	6	_	5	2	
Phs Duration (G+Y+Rc), s		13.5			13.5		5.6	21.1		7.4	22.9	
Change Period (Y+Rc), s		6.0			6.0		4.0	6.5		4.0	6.5	
Vax Green Setting (Gmax), s		25.0			25.0		8.0	30.5		8.0	30.5	
Max Q Clear Time (g_c+l1), s		3.0			5.0		2.6	8.1	•	3.9	12.8	
Green Ext Time (p_c), s		1.1			1.0		0.0	3.6		0.1	3.5	
ntersection Summary												
-ICM 2010 Ctrl Delay			13.0							····		
HCM 2010 LOS			В			•						
Votes												

	4	44-	4	1	M. J.	4	
Phase Number	1	2	4	5	6	8	
Movement	NBL	SBTL	EBTL	SBL	NBTL	WBTL	
Lead/Lag	Lead	Lag		Lead	Lag	.,,,,,	
Lead-Lag Optimize		Ü			3		
Recall Mode	None	C-Min	None	None	C-Min	None	
Maximum Split (s)	12	37	31	12	37	31	
Maximum Split (%)	15.0%	46.3%	38.8%	15.0%	46.3%	38.8%	
Minimum Split (s)	11	32.5	30	11	31,5	31	
Yellow Time (s)	3	4.5	3	3.	4.5	3	
All-Red Time (s)	1	2	. 3	1	2	3	
Minimum Initial (s)	4	7	4	4	7	4	
Vehicle Extension (s)	3	3	3	3	3	3	
Minimum Gap (s)	3	3	3	. 3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	Ō	
Walk Time (s)		7	. 7		7	7	
Flash Dont Walk (s)		19	17		18	18	
Dual Entry	No	Yes	Yes	No	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	. 7	19-	56	7	19	56	
End Time (s)	19	5 6	7	19	56	7	
Yield/Force Off (s)	15	49.5	1	15	49.5	1	
Yield/Force Off 170(s)	15	30,5	64	15	31.5	63	
Local Start Time (s)	31	43	0	31	43	0	
Local Yield (s)	39	73.5	25	.39	73.5	25	
Local Yield 170(s)	39	54.5	. 8	39	55.5	7	
Intersection Summary					,		
Cycle Length			80				
Control Type	Actua	ted-Coor	dinated				
M. I. I.O. I			•				

Offset: 56 (70%), Referenced to phase 2:SBTL and 6:NBTL, Start of Red Splits and Phases: 3: Rock Creek & Ziegler

Natural Cycle

epine and madoo.	o. Hour Grock a zieglei
↑ ø1	₩ a2 (R)
ø5	₹ Ø8

Intersection							٠			
Intersection Delay, s/veh	1.8			·						
Movement	EBL	EBT			WBT	WBR	SBL	SBR		
Vol, veh/h	57	89			144		3	7		
Conflicting Peds, #/hr	0	0			0		0	0		
Sign Control	Free	Free			Free		Stop	Stop		
RT Channelized		None					- Clop	None		
Storage Length	_	_				110110	. 0	NORE		
Veh in Median Storage, #	_	0			0	<u> </u>	0	=		
Grade, %	_	0			0		0	-		
Peak Hour Factor	85	90			85	85	85	- 85		
Heavy Vehicles, %	2	2			2		2			
Mvmt Flow	67	99			169	11	4	2 8		
Major/Minor	Major1				Major2		Minor2			
Conflicting Flow All	180	0			majorz	0	408	175		
Stage 1	-	-				-	175	1/5		
Stage 2		_					233			
Follow-up Headway	2.218	. ·				_	3.518	3.318		
Pot Capacity-1 Maneuver	1396	_				_	599	3.316 868		
Stage 1	-	_			_	_	855	808		
Stage 2	_	·-			_	-	806			
Time blocked-Platoon, %		_			_	-	000	, -		
Mov Capacity-1 Maneuver	1396	_		•			568	000		
Mov Capacity-2 Maneuver	-	_			_		568	868		
Stage 1	·	_			_	-	855	-		
Stage 2	·	=			-		765			
Ànnraeah	ĆD.									
Approach	EB				WB		SB			
HCM Control Delay, s	3			•	0		10			
Minor Lane / Major Mvmt		EBL	EBT	WBT	WBR	SBLn1		÷		
Capacity (veh/h)		1396				749				·
HCM Lane V/C Ratio		0.048	_	-		0.016			•	
-ICM Control Delay (s)		7.709	0	_	_	9:9				
-ICM Lane LOS		A	Å	_	-	9.9 A				
HCM 95th %tile Q(veh)		0.151	· -	-		0.048				
Notes				•				•		

^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Major/Minor Major1 Major2 Minor2 Conflicting Flow All 155 0 - 0 381 154 Stage 1 - - - 154 - Stage 2 - - - 154 Follow-up Headway 2.218 - - 227 - Follow-up Headway 2.218 - - 3518 3,318 Pot Capacity-1 Maneuver 1425 - 621 892 Stage 1 - - 874 - Stage 2 - - 811 - Fime blocked-Platoon, % - - 615 892 Mov Capacity-1 Maneuver 1425 - 615 892 Mov Capacity-2 Maneuver - - 615 - Stage 1 - - 874 - Stage 2 - - 804 - Approach EB WB SB HCM Control Delay, s 0 10	·····				·······························			<u> </u>			
Movement EBL EBT							_	•		•	
Vol. veh/h	ntersection Delay, s/veh	1.8									
Vol, veh/h Conflicting Peds, ##hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	EBL	EBT	• •		WBT	WBR	SBI	SBR		
Conflicting Peds, #hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Vol. veh/h	10									
Sign Control Free Free Free Free Free Free Free Stop Stop RT Channelized - None - Stop	•										
RT Channelized - None - None - One -											
Storage Length		-						Otop	•		
Veh in Median Storage, #		-				_	-	'n	NONE		
Crade, % 0 0 0 0 0 0 0 0 0		-	0			0			_		
Peak Hour Factor 85 86 95 85 85 85 85 85 85 85 85 86 86 95 85 85 85 85 85 85 85 85 85 85 85 85 85		_									
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		85				_	85		85		
Major/Minor Major1 Major2 Minor2	Heavy Vehicles, %										
Major/Minor Major1 Major2 Minor2	Mvmt Flow										
Conflicting Flow All 155 0 - 0 381 154 Stage 1 154 154 154 Stage 2 227 227 227 Follow-up Headway 2.218 3.518 3.318 Pot Capacity-1 Maneuver 1425 - 621 892 Stage 1 874 - 874 - 874 Stage 2 874 - 811 811 - 7 Stage 2 615 892 Mov Capacity-1 Maneuver 1425 - 615 892 Mov Capacity-1 Maneuver 1425 - 615 892 Mov Capacity-2 Maneuver 1425 - 615 892 Mov Capacity-2 Maneuver - 615 - 874 - 874 - 874 - 874 - 874 - 875 Stage 1 874 - 874 - 874 - 875 Stage 2 804 - 874 - 875 Stage 2 804 - 875 Approach EB WB SB HCM Control Delay, s 0 0 10 Minor Lane / Major Mymt EBL EBT WBT WBR SBLn1 Capacity (velv/h) 1425 - 788 HCM Lane V/C Ratio 0.008 - 0.087 HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 - 0.284							<i>f</i> _	40	40		
Conflicting Flow All 155 0 - 0 381 154 Stage 1 154 Stage 2 227 Follow-up Headway 2.218 3.518 3.318 Pot Capacity-1 Maneuver 1425 621 892 Stage 1 874 - Stage 2 874 - Stage 2 811 - Stage 2 615 892 Mov Capacity-1 Maneuver 1425 615 892 Mov Capacity-2 Maneuver 615 892 Mov Capacity-2 Maneuver 874 - Stage 1 804 - Stage 2 804 - Stage 3 804 - Stage 4 804 - Stage 5 804 - Stage 6 804 - Stage 7 804 - Stage 8 804 - Stage 9 80	Vlajor/Minor	Major1				Major2		Minor2			
Stage 1	Conflicting Flow All	155	0				0		154		
Stage 2	Stage 1	-	-						. 101		
Follow-up Headway 2.218 3.518 3.318 Pot Capacity-1 Maneuver 1425 621 892 Stage 1 874 - 811 811 - 811	Stage 2	· -,				· -	_		_		
Stage 1	Follow-up Headway	2.218	-			_	-		3.318		
Stage 1	Pot Capacity-1 Maneuver	1425	_			_	_				
Stage 2	Stage 1	-	_			. =	_		-	•	
Firme blocked-Platoon, %	Stage 2	; -	-			_					
Mov Capacity-2 Maneuver 615 Stage 1 874 Stage 2 804 Approach EB WB SB HCM Control Delay, s 0 0 10 Minor Lane / Major Mvmt EBL EBT WBT WBR SBLn1 Capacity (veh/h) 1425 788 HCM Lane V/C Ratio 0.008 0.087 HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 0.284	Firme blocked-Platoon, %		_			_	_				
Mov Capacity-2 Maneuver 615 Stage 1 874 Stage 2 804 Approach EB WB SB HCM Control Delay, s 0 0 10 Minor Lane / Major Mvmt EBL EBT WBT WBR SBLn1 Capacity (veh/h) 1425 788 HCM Lane V/C Ratio 0.008 0.087 HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 0.284	Mov Capacity-1 Maneuver	1425	~			_	_	615	892		
Stage 1 - - - 874 - 804 - 804 - - 804 - - 804 - - 804 - - 804 - - 804 - - 804 - - - 804 - - - 804 - <t< td=""><td>Mov Capacity-2 Maneuver</td><td>_</td><td>-</td><td></td><td></td><td></td><td>_</td><td></td><td>002</td><td></td><td></td></t<>	Mov Capacity-2 Maneuver	_	-				_		002		
Stage 2	Stage 1	_	_			_			_		
Approach EB WB SB HCM Control Delay, s 0 0 10 Minor Lane / Major Mvmt EBL EBT WBT WBR SBLn1 Capacity (veh/h) 1425 788 HCM Lane V/C Ratio 0.008 0.087 HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 0.284		-				_	_		_		
Compact Control Delay, s 0 0 10 10	-							55,			
Alinor Lane / Major Mvmt EBL EBT WBT WBR SBLn1 Capacity (veh/h) 1425 - - 788 ICM Lane V/C Ratio 0.008 - - 0.087 ICM Control Delay (s) 7.547 0 - 10 ICM Lane LOS A A B ICM 95th %tile Q(veh) 0.025 - - 0.284						WB		SB			
Capacity (veh/h) 1425 - - 788 HCM Lane V/C Ratio 0.008 - - 0.087 HCM Control Delay (s) 7.547 0 - - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 - - 0.284	HCM Control Delay, s	0				0		10			
Capacity (veh/h) 1425 - - 788 HCM Lane V/C Ratio 0.008 - - 0.087 HCM Control Delay (s) 7.547 0 - - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 - - 0.284	Minor Lane / Major Mvmt		EBL	EBT	WBT	WBR	SBLn1				
HCM Lane V/C Ratio 0.008 0.087 HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 0.284	· · · · · · · · · · · · · · · · · · ·			_		-				-	
HCM Control Delay (s) 7.547 0 - 10 HCM Lane LOS A A B HCM 95th %tile Q(veh) 0.025 0.284				_	_	-					•
ICM Lane LOS A A B ICM 95th %tile Q(veh) 0.025 0.284				0	_	-					
1CM 95th %tile Q(veh) 0.025 0.284											
				-	_	_					
							V.20T				

Interception												
Intersection Delay, s/veh	9.7							· ·				
Intersection LOS	9.7 A											
Movement	EBL.	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Vol, veh/h	52	24	16	8	83	51	36	94	3	27	59	34
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	0.00	
Mvmt Flow	61	28	19	9	98	60	42	111	4	32	69	40
Number of Lanes	0	1	1		1.	0	0	1	0	1	1	40 10
Approach	EB			WB			NB				÷	
Opposing Approach	WB			EB		_		···········		SB		· <u>-</u>
Opposing Lanes	1			2			SB			NB		
Conflicting Approach Left	SB			NB			2			1	-	
Conflicting Lanes Left	2						EB			WB		
Conflicting Approach Right	NB			1 SB			2			1		
Conflicting Lanes Right	1			.2			WB			EB		
HCM Control Delay	9.3			10.1			40.0			2		
HCM LOS	J.5			10.1 B			10.3			8.9		
TOMESO	^			Ь			В			Α		
Lane		NBLn1	EBLn1	EBLn2	WBLn1	SBLn1	SBLn2			,		
Vol Left, %		27%	68%	0%	6%	100%	0%					
Vol Thru, %		71%	32%	0%	58%	0%	63%					
Vol Right, %		2%	0%	100%	36%	0%	37%		-			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol.by Lane		133	, 76	16	142	27	93					
LT Vol		94	24	0	83	0	59	•				
Through Vol		3	0	16	51	0	34					
RT Vol		36	52	0	8	27	0					
Lane Flow Rate		156	89	19	167	32	109					
Geometry Grp		6	7	7	6	7	7					
Degree of Util (X)		0.239	0.145	0.025	0.245	0.053	0.158					
Departure Headway (Hd)		5.499	5.853	4.803	5.273	5.95	5.188					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Cap		649	609	739	677	599	687					
Service Time		3.567	3:626	2.575	3.34	3.718	2.956	•				
HCM Lane V/C Ratio		0.24	0.146	0.026	0.247	0.053	0.159					
ICM Control Delay		10.3	9.6	7.7	10.1	9.1	8.9					
HCM Lane LOS		В	Α	Α	В	Α	A					
HCM 95th-tile Q		0.9	0.5	0.1	1	0.2	0.6					
Notes												

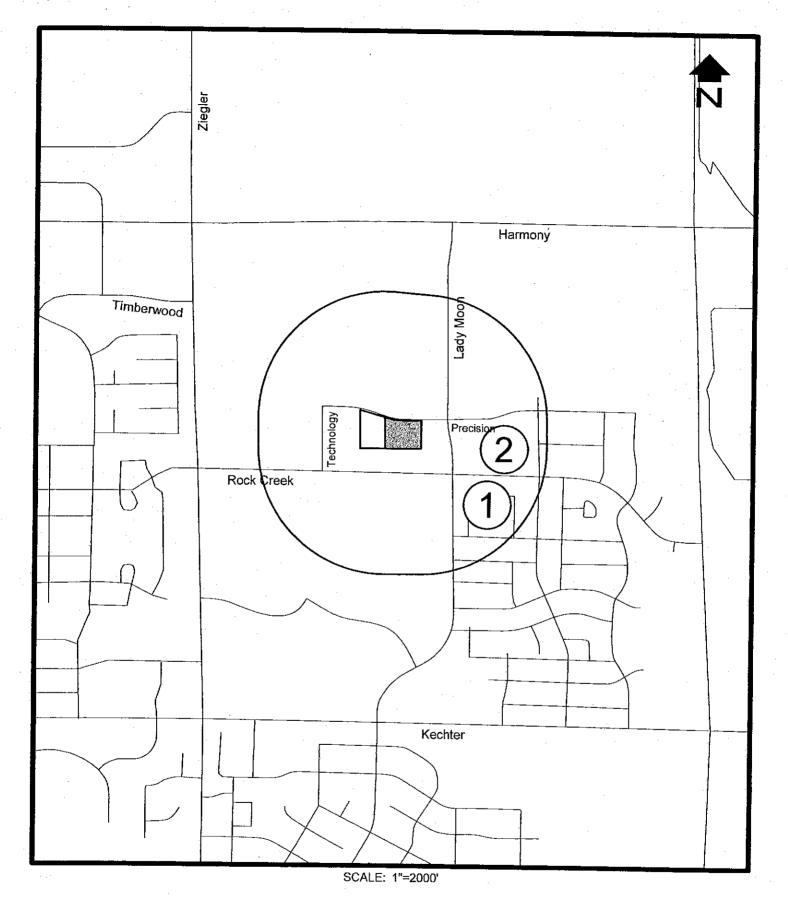
^{~:} Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Intersection												
Intersection Delay, s/veh	10.1								 			
Intersection LOS	В											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	CDE
Vol, veh/h	41	- 86	65	3		30	35	61	6	50		SBR
Peak Hour Factor	0.85	0.85	0.85	0.85		0.85	0.93	0.85	0.85	0.85	126 0.85	44
Heavy Vehicles, %	2	2	2	. 2		2	2	2	2	2		0.85
Mvmt Flow	48	101	76	4	76	35	38	72	7	59	2 148	2
Number of Lanes	0	1	1	0	1	0	0	1	0	1	140	52 0
Approach	EB			WB			NB		÷	0.0		
Opposing Approach	WB			EB				 		SB	<u> </u>	<u>. </u>
Opposing Lanes	1			2			SB			NB		
Conflicting Approach Left	SB			NB			2			1		
Conflicting Lanes Left	2			110			EB			WB		
Conflicting Approach Right	NB			SB			2			1		
Conflicting Lanes Right	1			2			WB			EB	•	
HCM Control Delay	9.8			10.1			100			2		
HCM LOS	A			В			10.3	-		10.3		
						•	В			В		
Lane		NBLn1	EBLn1	EBL:n2	WBLn1	SBLn1	SBLn2					
Vol Left, %		34%	32%	0%	3%	100%	0%				····	
Vol Thru, %	•	60%	68%	0%	67%	0%	74%			•		
Vol Right, %		6%	0%	100%	30%	0%	26%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		102	127	65	101	50	170					
LT Vol		61	86	0	68	0	126					
Through Vol		6	0	65	30	0	44					
RT Vol		35	41	0	3	50	0					
_ane Flow Rate		116	149	76	115	59	200					
Geometry Grp		6	7	. 7	6	7	7					
Degree of Util (X)		0.192	0.242	0.106	0.185	0.099	0.299					
Departure Headway (Hd)		5.926	5.938	5.068	5.774	6.18	5.493	-				
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Cap	•	608	608	711	625	584	658					
Service Time		3.934	3.638	2.768	3.782	3.88	3.193					
ICM Lane V/C Ratio		0.191	0.245	0.107	0.184	0.101	0.304					
		10.3	10.5	8.4	10.1	9.6	10.5					
ICM Control Delay			10.0									
ICM Lane LOS		. B	В	A	В	Α	В					
•								÷				

Intersection						-						
Intersection Delay, s/veh	2.1						<u>-</u>					
Movement	EBL	EBT	EBR	WBL	-WB1	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Vol, veh/h	7	0	1	13	1		4	191	2	11	106	29
Conflicting Peds, #/hr	0	. 0	0	0			0	0	0	0	0	·Z:
Sign Control	Stop	Stop	Stop	Stop		_	Free	Free	Free	Free	Free	Free
RT Channelized	~	-	None				-		None	1100	1100	None
Storage Length	-	-	_	_			150	_	140110	150	-	NODE
Veh in Median Storage, #	-	0	-		. 0	_	-	0	_	150	^	
Grade, %	_	0	_	· _			_	0	-	_	0	
Peak Hour Factor	85	85	85	85	85		85	85	85	- 85	0	
Heavy Vehicles, %	2	2	2	2	. 2	_	2	2.	2		85	85
Mvmt Flow	8	0	1	15	1	55	5	225	2	2 : 13	125	2 34
Major/Minor	Minor2		٠	Ňůa				* *				
				Minor1			Major1	<u> </u>		Major2		
Conflicting Flow All	432	404	142	403	420	226	159	0	0	227	0	0
Stage 1	168	168	-	235	235	-	-	-		_		
Stage 2	264	236		168	185		-	-	-		· -	_
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-		2.218	_	_
Pot Capacity-1 Maneuver	534	536	906	558	525	813	1420	-	-	1341		
Stage 1	834	759	-	768	710	-	-	~	_ ·.	-	_	_
Stage 2	741	710		834	747	-	-	-	_		_	
Time blocked-Platoon, %								-				_
Mov Capacity-1 Maneuver	492	529	906	552	518	813	1420	-	-	1341	_	_
Mov Capacity-2 Maneuver	492	529	-	552	518	_	-	_		-	_	_
Stage 1	831	752	-	765	708	_	_	_	_	_	_	
Stage 2	687	708		825	740	-	-	-	-		-	-
Approach .	EB			WB			NB			SB		
ICM Control Delay, s	12			10			0			1		
Minor Lane / Major Mymt		NDL	NDT	·	-n.							
		NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1420	-		522	732	1341	-	-			
ICM Lane V/C Ratio		0.003	-	-	0.018	0.098	0.01	-	-			
ICM Control Delay (s)		7.544	-	<u>-</u>	12	10.5	7.711		-			
ICM Lane LOS		Α			В	В	Α					
ICM 95th %tile Q(veh)	•	0.01	-	-	0.055	0.325	0.029	-	-			
lotes												
: Volume Exceeds Capacity	r 🛊 · Delay	Evenado	200 000									

Intersection					-							
Intersection Delay, s/veh	2.5								· · · · ·	·	·	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	35	1	4	6	0	22	0	119	13	39	210	7
Conflicting Peds, #/hr	0	0	0	0	.0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	_		None		-	None
Storage Length	-	-	-		-	_	150	_	-	150	_	
Veh in Median Storage, #	-	0	-	-	0	_	_	0	••	-	0	
Grade, %	• -	0	-	-	.0	-	-	0	-	_	Ö	
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	41	1	5	7	0	26	0	140	15	46	247	8
												J
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	504	498	251	494	495	148	255	0	0	155	0	0
Stage 1	343	343		148	148	-		_	-	100	_	U
Stage 2	161	155	-	346	347	_			_		_	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218		-
Pot Capacity-1 Maneuver	478	474	788	486	476	899	1310	· · <u>-</u>	_	1425	•	-
Stage 1	672	637		855	775	-	-	_		1720		-
Stage 2	841	769	_	670	635	_	_	_	_		_	-
Time blocked-Platoon, %							-	_			_	
Mov Capacity-1 Maneuver	453	459	788	470	461	899	1310	_		1425		
Mov Capacity-2 Maneuver	453	459	-	470	461		_		_	-	_	•
Stage 1	672	616	_	855	775	_	_	_	_	_	_	_
Stage 2	817	769	-	643	615		_	_	_	_	-	_
				*		٠						_
Approach,	EB			WB			NB			SB		
HCM Control Delay, s	14			10	· · · · · · · · · · · · · · · · · · ·		0			 1		
							~			'		
Minor Lane / Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1310			473	752	1425	ODT	ODIT			
HCM Lane V/C Ratio			_	_	0.099	0.044	0.032	-	-			•
HCM Control Delay (s)		-0		~	13.5	10	7.61	-				
HCM Lane LOS		Ā			13.3 B	. B	7.01 A		-	•		•
HCM 95th %tile Q(veh)		0	· _		0.329	0.137	0.1	_				
Notes		-			5.52.0	0.101	0.1	_	_			
~ : Volumo Evenado Consoit	e . DI	. F	000.0	anda F-		 .	1.15.6		 <u>-</u>	<u> </u>		

APPENDIX F



PEDESTRIAN INFLUENCE AREA

Pedestrian LOS Worksheet

Project Location Classification: OTHER

	Description of	Destination		Level of Service (minimum based on project location classificat									
	Applicable Destriction Area Within 1320	Area Classification		Directness	Continuity	Street Grossings	Visual Interest & Amenities	Securi					
	OBSGRUATORY		Minimum		C	ے	ے	<					
1	1	RESIDEUM	Actual	A	В	B	ß	B					
	VILLAGE		Proposed	A	В	В	В	В					
	PRESIDIO	D	Minimum	2	C	J	U						
2	Aprs	RESIDENTIAL	Actual	A.	B	В	В	B					
	4113		Proposed	A	В	<u>8</u>	В	8					
			Mininum					×					
3			Actual										
			Proposed										
_	. •	-	Minimum										
Ц			Actual					·					
			Proposed										
_			Minimum										
			Actual										
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L			Proposed										
4			Minimum										
4			Actual					· · · · ·					
\perp			Proposed										
4		<u>[:</u>	Minimum					······································					
1			Actual										
_			Proposed			· ·							
1			Minimum										
-			Actual										
L			Proposed										
			Minimum										
1			Actual					·					
L	<u> </u>		Proposed										



July 9, 2013

Planning, Development and Transportation Planning 281 North College Ave. P.O. Box 580 Fort Collins, CO 80522-0580 970.221.6750 970.224.6134 - fax fcgov.com/developmentreview

Dear Property Owner:

On **Tuesday**, **July 23rd**, **2013**, **at 5:00 p.m.** (or shortly thereafter), in Conference Room A at 281 North College Avenue, Fort Collins, Colorado, a City of Fort Collins Hearing Officer will conduct an administrative public hearing to consider a development proposal in your neighborhood. The project is referred to as Harmony Technology Park, third Filing, Second Replat – Custom Blending, Combined Project Development Plan/Final Plan - FDP #130021.

This request is for a combined Project Development Plan/Final Plan for a building expansion to the Custom Blending facility located at 3461 Precision Drive. Custom Blending is a manufacturer of spices, seasonings and flavoring extracts. The impetus for the expansion is to create space primarily for the manufacture of extracts in-house. The approximate 31,700 square foot existing Custom Blending building is located on 3 acres, in Lot 2 in the Harmony Technology Park Third Filing. The proposed addition to the east of the existing building would contain approximately 35,000 square feet, and expand into a portion of Lot 3. This project will include re-platting to combine Lot 2 and Lot 3. The proposed expansion is anticipated to be 40-50% production/warehouse and approximately 3,000 to 5,000 square feet of office space.

The proposed light industrial uses are permitted in the Harmony Corridor (HC) zone district, subject to administrative review and public hearing. Additional information, including the subdivision plat, site plan, and building elevations (provided by the applicant), can be found at: www.fcgov.com/developmentreview/agendas.php.

The City's Planning Staff and Hearing Officer consider your interest and input in this matter an important part of the City's review of the proposal. If you are unable to attend the public hearing, but would like to provide input, written comments are welcome via U.S. mail to the address above or you may e-mail me at pwray@fcgov.com.

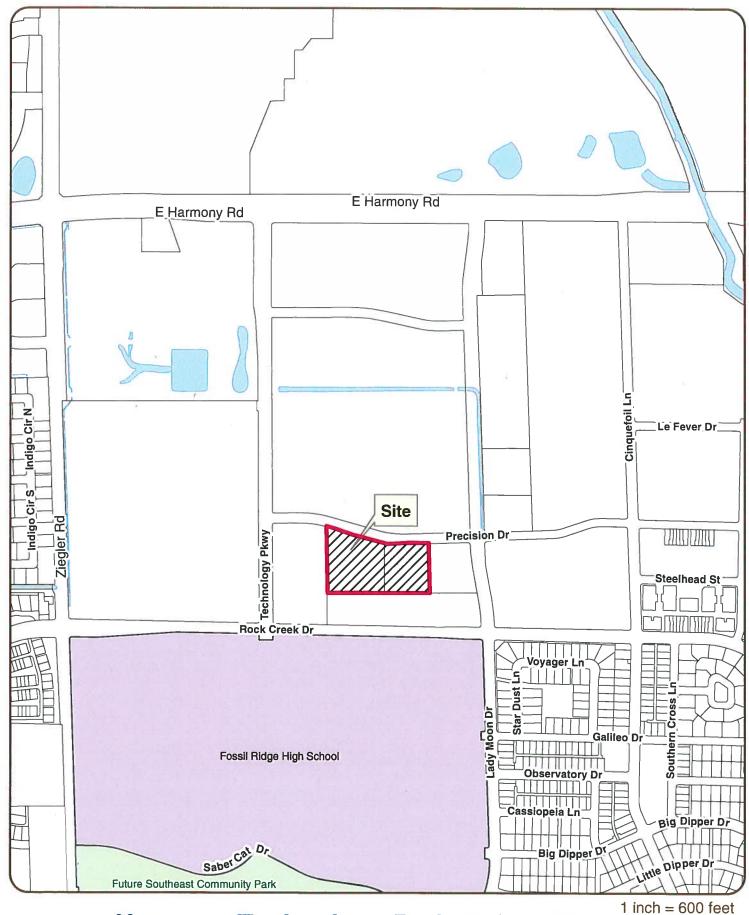
The notification mailing list for this public hearing is derived from Larimer County Assessor records. Because of the lag time between home occupancy and record keeping, or because of rental situations, a few affected property owners may have been missed. Please feel free to notify your neighbors of this hearing so all may have the opportunity to attend.

Sincerely

Pete Wray, AICP, Senior City Planner

Project Planner

The City of Fort Collins will make reasonable accommodations for access to City services, programs, and activities and will make special communication arrangements for persons with disabilities. Please call (970) 221-6750 for assistance.



Harmony Technology Park, Third Filing, Second Replat - Custom Blending

